ADDENDUM NO. TWO

TO

BID DOCUMENTS, CONTRACT DOCUMENTS,
CONSTRUCTION SPECIFICATIONS AND DRAWINGS

FOR

PUBLIC WORKS VEHICLE MAINTENANCE FACILITY

FOR THE

CITY OF SOCIAL CIRCLE, GEORGIA

PROJECT NO. 242774

Bids Received Until Thursday, APRIL 23, 2026 at 10:00 a.m.

ACKNOWLEDGE RECEIPT OF THIS ADDENDUM BY INSERTING ITS NUMBER IN THE PROPOSAL. FAILURE TO DO SO MAY SUBJECT BONA FIDE BIDDERS TO DISQUALIFICATION. THIS ADDENDUM FORMS A PART OF THE PROJECT DOCUMENTS; IT MODIFIES THEM AS FOLLOWS:

April 17, 2026

TURNIPSEED ENGINEERS

Atlanta ▪ 2255 Cumberland Parkway ▪ Building 400 ▪ Atlanta, Georgia 30339 ▪ Tel: 770-333-0700
Augusta ▪ 4210 Columbia Road ▪ Building 3 ▪ Augusta, Georgia 30907 ▪ Tel: 706-863-8800

FLEET FACILITY - CONSTRUCTION SPECIFICATIONS

Refer to Fleet Facility Project Manual, Index of Specifications

Replace with attached Index of Specifications

Refer to Section 10-2813 Mirror Units

Insert attached Section 10-2113, Toilet Compartments in front of Section 10-2813.

NEW FLEET FACILITY - CONSTRUCTION DRAWINGS

Refer to New Fleet Facility, Architectural Plan Sheet A4.01.

Replace with attached Architectural Plan Sheet A4.01.

Refer to New Fleet Facility, Architectural Plan Sheet A5.01.

Replace with attached Architectural Plan Sheet A5.01.

Refer to New Fleet Facility, Structure Plan Sheet S0.01.

Replace with attached Structure Plan Sheet S0.01.

Refer to New Fleet Facility, Structure Plan Sheet S2.01.

Replace with attached Structure Plan Sheet S2.01.

PUBLIC WORKS BUILDING – LAND DISTURBING PLAN - CONSTRUCTION DRAWINGS

Refer to Public Works Building – Land Disturbing Plan, Plan Sheet E-6.

Replace with attached Plan Sheet E-6, Water & Sewer Utility Plan

Refer to Public Works Building – Land Disturbing Plan, Plan Sheet E-7.

Replace with attached Plan Sheet E-7, Miscellaneous Details

Refer to Public Works Building – Land Disturbing Plan, Plan Sheet E-8.

Replace with attached Plan Sheet E-8, Initial SESC Plan

BID QUESTIONS / CLARIFICATIONS

1. **Please confirm if an AISC fabricator/installer is required for the steel on this project.-**

   Both the fabricator and the erector shall be AISC-certified. See Spec Section 05 1200-Structural Steel Framing, Sections 1.05 A & B.

2. **There does not appear to be any specification sections after division 28 such as Earthwork, Exterior improvements, or Utilities. Please confirm if there are additional specs that are missing, or if the information on the civil drawings is the extent of the information for these divisions.**

   Refer to both architectural and civil drawings.

3. **Please confirm the install location of the Bird-B-gone netting, I do not see it on the drawings**

   Netting shall be on all covered equipment storage buildings and the wash bay of the Fleet Facility. Every exposed roof to the exterior.

4. **On the architectural drawings there is equipment shown such as the pressure washer, air compressor, ice maker, shelving in parts and battery rooms, workbench, and furniture in the show up room. There doesn't appear to be an equipment schedule with a what is and isn't in our contract, and I don't see any specifications for these things either, please confirm.-**

   Equipment is owner provided.

5. **For the overhead coiling doors, the specifications say that substitutions will be allowed if they match the basis of design in form and function, we have had a few subcontractors reach out asking if any of the following manufacturers will be accepted in place of overhead door company for this bid, Raynor, Wayne Dalton (a division of Overhead Door), CHI, Clopay, or**

Janus? Would the subcontractor need to fill out any special substitution request form for this?-

No

6. **S2.01 has 8 EA F7 spread footers labeled but schedule at top of page only lists F4, F6, & F8. I just wanted to verify if that footer needed to be F7 or F8, and if F7, verify that it's 7' x 7' x 2'. –**

   F7 footings to be 7'x7'x2'. Revised sheet S2.01 to be issued.

7. **Elevation 6 on A4.01 is calling for a 4' high painted CMU wall. I don't see that shown on floor plan.**

   Disregard reference to 4' high painted CMU wall.

8. **Sheet A6.01 shows the restroom walls, at least the "wet" walls, getting tile, or it looks like tile in the elevations. The finish schedule says "paint" for walls. If it is tile, we don't have tile specs.**

   Painted CMU for the walls.

9. **In those same restrooms, is that a 4" CMU behind the toilets & sinks to create the chase? I don't see that it's labeled.**

   Yes 4" CMU.

10. **Where are the toilet compartment specifications?**

    See spec. section 102113

11. **It looks like the interior signage specifications, section 10 1467 (page 5) don't correspond with this job. Can we get clarification on interior signage requirements?**

    Men's Restroom, Women's Restroom, 3 evacuation plan frames.

12. **In the specs they are calling for a AVP wall panel, we were wondering if we can submit a substitution request for either the similar wall panels offered by Varco Pruden and Nucor. I am attaching the data sheets for both wall panels.**

    Yes, please submit during shop drawing submittal.

13. **Is the mezzanine by others or do you want us to include this in our scope?**

    The mezzanine is included in the scope of this project. General Contractor shall coordinate supplier.

14. **In the drawings it mentions Spray Foam insulation. Spray foam insulation can be great in certain structures, but it really does not work well with PEMB's – I wanted to confirm that spray foam will be used and we need to load for it, or do you want a fiberglass alternative?**

    Yes, use spray foam insulation.

15. **Our suppliers have their standard girt's at 8 or 8 ½ I wanted to confirm this is sufficient and acceptable.-**

    Design of girt sizes and spacing is the responsibility of the PEMB supplier.

16. **I wanted to clarify if, red oxide, gray primer or galvalume is the required finish and if it all needs to be the same. One of our suppliers has gray primer for their primary frames but their secondaries are G30 galvanized.**

    Gray Primer for everything.

17. **On the roof only structures it appears to have a 1' overhang – Please confirm if the overhang requires a soffit.**

    Yes, it does.

18. **There is no collateral load noted, can you confirm if a 3# PSF is sufficient?**

    The design loads on S0.01 include +5 psf roof load for miscellaneous (in addition to the building self weight). This +5 psf may be considered the minimum acceptable collateral load for design.

19. **The specs mention a "Bird B Gone" netting system, in lieu of that system, can we substitute this for an interior liner panel? This would completely enclose the roof and would not allow birds to nest in the roof cavity.**

    Yes, submit proposed panel.

20. **Is the portable generator to be supplied by contractor or supplied by others?-**

    Provided by others.

21. **IBC 2024 was adopted statewide in GA as of Jan 2026, S0.01 reference IBC 2018. Please confirm this is grand fathered in on 2018.**

    IBC 2024 is correct. Revised sheet S0.01 to be issued will indicate IBC 2024.

22. **S0.01 – please confirm note 10 is accurate and requires structural jambs and headers for roll up and overhead doors. Sizes to be per PEMB design to fit within wall cavity.**

    Note 10 is correct.

23. **Confirm the storage mezzanine will be independent of the PEMB.**

    Storage mezzanine does not rely on PEMB for vertical or lateral support.

24. **S3.03 detail 10 – please confirm the sized angle attaching from the CMU to the PEMB is not by the building supplier, Loading only shown in detail is to be accounted for.**

    Contractor shall coordinate who provides various steel elements.

25. **Please confirm all sized steel is not by the PEMB supplier.**

    Contractor shall coordinate who provides various steel elements.

26. **A3.01 Detail 6 – reference Mtl building insulation typ. Please confirm spray foam will be used per wall sections on A5.01**

    Spray foam

27. **Please elaborate on the 10"+/- dimension referenced on A5.01 details 1 and 2. An 8" girt is shown within a 10" cavity. It looks like 10" girts will be required to go from face of flange to back of panel and keep everything flush at the CMU with hats location. Please advise.**

    10" is to the column line and there is plywood lining on interior up to 10' A.F.F.

28. **Please confirm the roof overhang dimension on the covered equipment storage. –**

    +/- 2'-9", can vary by manufacturer.

29. **Please list which portal frames detail 6 on S3.02 apply.**

    All columns on sheet S1.01 Gridline A, and all columns on S2.01.

30. **Section 03 300 3.14 doesn't mention what type of sealer/densifier to use. Can you please clarify?**

    Refer to Architectural drawing A2.01 finish schedule note #2.

31. **Should all floor joints be filled in the sealed concrete areas, and if so, what type of material?**

    Yes, fill joints with an elastomeric joint filler/sealant per Structural drawings.

32. **Section 3 300 3.11F mentions that broom finish should be used in exterior areas with vehicular traffic. Does this apply to rooms 101 and 113 and if so, should they not be grinded and sealed since that would remove the broom finish?-**

    Do not grind and seal 101 and 113.

33. **E2.01- KeyNote 2 " refer to detail ??/??? For additional information." No detail shown and no additional information.**

    KEYNOTE-2 has been revised to say the following: “CIRCUIT VIA LIGHTING CONTACTOR – REFER TO DETAIL 2/E6.00 FOR ADDITIONAL INFORMATION.” and will be indicated on the next Addendum.

34. **E2.01, KeyNote 7 " refer to detail ??/??? For additional information. " No detail shown and no additional information.**

    KEYNOTE-7 has been revised to say the following: “MASTER OVERRIDE SWITCH – REFER TO DETAIL 2/E6.00 FOR ADDITIONAL INFORMATION.” and will be indicated on the next Addendum.

35. **E3.01- General note 2 " Refer to voice/data drawings. " No drawings found.**

    GENERAL NOTE-2 has been revised to say the following: “IT LAYOUT IN STORAGE MEZZ. ROOM IS SCHEMATIC IN NATURE AND DOES NOT INDICATE ALL COMPONENTS REQUIRED FOR A COMPLETE VOICE/DATA SYSTEM – REFER TO OWNER PROVIDED VOICE/DATA VENDOR DRAWINGS FOR COMPLETE RACEWAY REQUIREMENTS.” and will be indicated on the next Addendum.

36. **Main electrical service entrance is shown to be over head on E6.00 and underground on E3.01.is the main electrical service entrance over head or underground. If over head does this apply to phone service entrance and will it be over head? If so please specify conduit type, size and phone cabinet if needed?**

    It is PCE’s understanding that the Electrical Service will be provided from a Pole Mounted Transformer (location to be determined by the owner and utility company). However, the Electrical Secondary Service from the pole mounted

transformer shall be ran down the pole and underground to the utility meter located on the exterior of the building. Detail 1/E6.00 has been revised for the next Addendum.

37. **E3.01- note 3 " Provide open/close push buttons for motorized door." If we are to provide the open/close push button. Can you please give specifications on motor and control wiring?**

    KEYNOTE-3 has been revised to say the following: "PROVIDE RECESSED JUCTION BOX WITH 0.50" CONDUIT TO DOOR MOTORE FOR OPEN/CLOSE PUSH BUTTON WIRING FOR MOTORIZED DOOR AT 48" A.F.F. – COORDINATE EXACT REQUIREMENTS WITH MOTORIZED DOOR MANUFACTURER PRIOR TO ROUGH-IN. (TYPICAL)" and will be indicated on the next Addendum.

38. **E6.00- Portable generator docking station. So that generator portable station and generator are compatible, can you please give more description and/or specifications of type of generator required?**

    Please refer to KEYNOTE-9 and coordinate these requirements with the Owner.

39. **The specs calls for a voice fire system instead of horn/strobes? The drawing shows a horn/strobe system, please confirm**

    The Fire Alarm System will use horn/strobes. Specification Section 28 31 00 – FIRE ALARM SYSTEM has been revised to remove the 28 31 00 – 2.04 VOICE COMMAND CENTER requirement along with removing 28 31 00 – 2.07.N Speakers and will be issued for the next Addendum.

40. **Please provide details of the proposed dual wall dual sided 5000 gallon tank, and the concrete slab. Is the tank poured concrete? Is the tank pre manufactured? Please also provide details of the slab this tank sits on.**

    See sheet E-1.

41. **Please provide storm line and storm structure details and profiles.**

    All storm lines and structures are shown in the plan view drawings. No profiles provided.

42. **Is the Sd1-S layer double row?**

    Note on sheet E-8 for Sd1-S has been updated to reflect a double row of silt fence. See revised sheet E-8.

43. Do you want us to include a sanitary sewer cleanout going to the maintenance building from the proposed manhole?

The label shown on sheet E-6 has been updated to reflect a cleanout at the location just outside of the building. See revised sheet E-6.

44. Please provide storm and sewer bedding details. Sewer line in wetland detail provided but no proposed sewer is shown in the wetland.

Pipe bedding detail has replaced the sewer line in wetland detail on sheet E-7. See revised sheet E-7.

45. On sheet E9 there is a callout for "prop. Rock rip-rap" but the callouts do not point to anything. Please elaborate.

Note is referencing the rip rap weir that is the discharge for the stormwater detention pond.

46. Please clarify extents of heavy and light duty asphalt paving accordingly.

Light duty pavement will be utilized in the north parking area (20 space lot) and heavy duty will be utilized everywhere else.

47. Provide waste characteristics for debris removal area? Is waste non-hazardous?

Buried debris is household waste.

48. Is the intent to balance the dirt onsite to avoid haul in or haul off?

Social Circle owns the property adjacent to this site. Any excess will be able to be placed there.

49. Does the city have a place nearby to haul the excess dirt?

See answer to 21.

50. It looks like the Addendum 1 Grading and Drainage sheet E-5 is still missing some of the storm pipe labels(size and material).

Addendum 1 provided sheet E-5.

51. **It appears the fuel tank specification is calling for a stand alone meter with a hose reel. This is used primarily in airport installs . In most fleet facilities a commercial type fueling dispenser Is used that can tie into a fuel management system if desired. Would the city consider this type of set up? I am attaching a cut sheet for a fleet dispenser.**

    Provide as called out in specifications.

52. **On drawing E3 there is both light and heavy-duty asphalt, is all of the pavement to be heavy duty except the 20 proposed parking area? Are the 23 proposed parking spots also to be light duty? Please confirm where light and heavy duty start and end.**

    See 19.

53. **We are seeing a pretty big export of soils on this job. There is a note in the plans that says haul off debris to landfill. Are the soils going there too?**

    Debris will be screened to ensure only waste goes to the landfill.

54. **My next question is actually about the haul off of debris. Is there any insight you can give about what type or how much debris there is?**

    See Geotechnical report provided.

55. **Will the project soils be handled as unclassified for unsuitable soils?**

    No.

56. **When submitting the qualification packet, do we need to include the Contract Agreement?**

    When submitting your bid please fill out the following documents:

00300	Bid Proposal
00410	Bid Bond
00420	Statement of Bidder's Qualifications
00480	Noncollusion Affidavit of Prime Bidder
00490	Noncollusion Affidavit of Subcontractor

    The Contract Agreement and other documents will be filled out by the awarded contractor.

57. **For the Noncollusion Affidavit of Subcontractor, we typically get all subs to sign once the project is awarded and we go to contract with the subs. If these are required to send in with the qualification packet, will I need to have all subs that are bidding the project sign and submit this form?**

    This can be turned in with Contract Documents, if awarded.

58. Is the generator, MTS and the docking station existing? If we are required to furnish these items, can you please provide part and model # for these?

To be addressed with Addendum 3.

59. In Door Hardware Specification, 1.02D it mentions allowances. section 012100 Allowances appears to be missing please advise.

To be addressed with Addendum 3..

END OF ADDENDUM NO. TWO

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SECTION 10 2113

TOILET COMPARTMENTS

PART 1 - GENERAL:

1.01 RELATED DOCUMENTS:

A. Drawings and general provisions of the Contract, including General and Supplementary Conditions and Division 1 Specification Sections, apply to this Section.

1.02 WORK INCLUDED:

A. Phenolic Toilet Partitions.
B. Phenolic Urinal Screens.

1.03 RELATED SECTIONS:

A. Section 03 3000 – Cast-In-Place Concrete.
B. Section 06 1000 – Rough Carpentry.
C. Section 07 9200 – Joint Sealants.
D. Section 09 2000 – Gypsum Board.
E. Section 09 6723 – Resinous Flooring.
F. Section 10 2800 – Toilet & Bath Accessories.

1.04 REFERENCES:

A. American Society for Testing and Materials:
   1. ASTM E84 Standard Test Method for Surface Burning Characteristics of Building Material.
   2. ASTM D6578 Standard Practice for Determination of Graffiti Resistance.
   3. ASTM D1037 Direct Screw Withdrawal Test.
   4. ASTM D570 Standard Test Method for Water Absorption.
   5. ASTM A167, 18-8, Type 304 Cast Stainless Steel.

B. National Fire Protection Association (NFPA), latest adopted edition with Georgia Code Amendments.

C. ADA, Accessibility Guidelines for Buildings and Facilities.

D. International Building Code, latest adopted edition with Georgia Code Amendments.

E. 2005 LD-3 NEMA Standard Test, Chemical Resistance, Modulus of Elasticity, Shear Strength and Compression Strength.

1.05 QUALITY STANDARDS:

A. Flame Spread: When tested in accordance with ASTM E84, Toilet Partition and Urinal Screen materials shall meet or exceed all requirements for Class A Flame Spread Rating and Smoke Developed and shall carry a Class A Fire Rating Certification in accordance with the requirements of NFPA and ICC. Class A Fire Rating Certification shall be in the name of the Toilet Partition Manufacturer and shall be less than six (6) months old.

1. Flame Spread shall not exceed 25.
2. Smoke Developed shall not exceed 450.

B. Graffiti Resistance Requirements: When tested in accordance with ASTM D6578, partition materials shall prove resistant to all chemicals tested for a period of 1 to 10 minutes and shall leave no mar or blemish on the surface when cleaned. Partition materials shall have guaranteed surface clean-ability from permanent markers and shall have Non-Ghosting properties.

C. Scratch Resistance Requirements: When tested in accordance with ASTM D2197, partition materials shall prove to be scratch resistant when the maximum Load Value exceeds 10 kilograms.

D. Impact Resistance Requirements: When tested in accordance with ASTM D2794, partition materials shall withstand an Impact Force Value in excess of 45 inch-lbs.

E. Screw Holding Strength: When tested in accordance with ASTM D1037, Direct Screw Withdrawal Test, partition materials shall withstand a direct pull force that exceeds 2,500 lbs per fastener.

F. Tensile Strength: Partition materials shall have a Modulus of Elasticity of 1.55 Million PSI.

G. Shear Strength: Partition materials shall have a Shear Strength of 2,000 PSI minimum.

H. Compression Strength: Partition materials shall have a Compression Strength of 24,000 PSI minimum.

I. Water Absorption Requirements: When tested in accordance with ASTM D570 partition materials shall have a Water Absorption Rate of less than 0.37%.

J. Door Hardware, Hinges and Mounting Brackets shall be 14 Gauge Type 304 Stainless Steel.

1.06 SUBMITTALS:

A. Product Data: Submit manufacturer's detailed technical data for materials, fabrication and installation, including catalog cuts of anchors, hardware, fasteners, and accessories in accordance with Section 01 3300 – Submittal Procedures.

B. Shop Drawings: Furnish Shop Drawings in quantities requested for fabrication and installation of Toilet Partitions and Urinal Screens assemblies. Include plans, elevations, sections, details and anchorages/attachments to other work.

C. Samples for Verification:
1. Submit 6" square Samples of each color and finish on same substrate used in work, for color verification after selections have been made.
2. Submit one (1) sample of the following:
   a. Hardware (Complete).
   b. Pilaster (12" X 12").
   c. Divider Panel (12" X 12").
   d. Aluminum Mounting Bracket.
   e. Stainless Steel Hinge.

D. Maintenance Instructions: Provide manufacturer's printed Instructions for Cleaning and Maintenance of Installed Work.

E. Manufacturer's Written Warranty: Provide manufacturer's Written Warranty as detailed herein.

1.07 PROJECT CONDITIONS:

A. Field Measurements: Verify dimensions in areas of installation by field measurements before fabrication and indicate measurements on Shop Drawings. Coordinate fabrication schedule with construction progress to avoid delaying the Work.

1.08 DELIVERY, STORAGE AND HANDLING:

A. Deliver materials in manufacturer's original packaging to protect from damage.

B. Store materials in manufacturer's original packaging in accordance with manufacturer's instructions. Store materials indoors, protected from the elements and construction hazards.

C. Handle materials in a manner that will protect the finished product.

1.09 MANUFACTURER'S WARRANTY:

A. Provide manufacturer's Twenty-Five (25) year written limited warranty on its Panels, Pilasters and Doors, against chipping, breakage, corrosion, delamination and defects in workmanship; to be replaced without charge excluding labor.

B. Provide manufacturer's Ten (10) year written limited warranty on all Cast Stainless Steel Hardware, Hinges and Mounting Brackets, as well as on all full high aluminum mounting brackets, against defects in material and workmanship. All other hardware will be provided with a manufacturer's One (1) year written limited warranty, against all defects in material and workmanship.

PART 2 - PRODUCTS:

2.01 MANUFACTURERS:

A. Manufacturer: Subject to compliance with requirements, provide products of one of the following :
   1. Series 72833; COLUMBIA PARTITIONS®, a Division of PSiSC®; P.O. Box 181, Columbia, SC 29202; Tel: 866-337-7286; Fax: 866-337-7291.
   2. Series 40; General Partitions Manufacturing Corporation; 1702 Peninsula Drive, Erie, PA 16505-4243; Tel.: 814-833-1154; Fax: 814-838-3473; Email: quotes@generalpartitions.com.
   3. Phenolic Black Core; Global Partitions, An ASI Group Company; 2171 Liberty Hill Road, Eastanollee, Georgia 30538; Tel.: 706-827-2700; Fax: 706-827-2710; Email: Sales@GlobalPartitions.com.

B. Section 01600 – Product Requirements: Product selection procedures: Substitution: Permitted.

2.02 INSTALLATION TYPES:

A. Toilet Partitions shall be:
   1. Overhead Braced, Floor Anchored.

B. Urinal Screens shall be:
   1. Wall-Hung.

2.03 MATERIALS:

A. Material shall be Class A Fire Rated Phenolic with a High Pressure Melamine matte finish surface made as an integral part of the core material. Laminated surfaces are not acceptable. Surface and edges shall be non-porous and shall not support fungus or bacteria. Provide material which has been selected for uniform color, surface flatness and smoothness. Exposed surfaces which exhibit discolorations, pitting, seam marks, roller marks, stains, telegraphing of core material, or other imperfections on finished units are not acceptable. Defects such as chipping along edges and corners are unacceptable. Class A Fire Rated Phenolic shall meet or exceed all requirements for Class A Flame Spread Rating and Smoke Developed calculated according to ASTM E84, and shall carry a Class A Fire Rating Certification. Class A Fire Rating Certification shall be in the name of the Toilet Partition Manufacturer and shall be less than six (6) months old.

B. Material Thicknesses:
   1. Doors: Minimum .75" Finished Thickness.
   2. Divider Panels: Minimum .75" Finished Thickness.
   3. Pilasters: Minimum 1.00" Finished Thickness.

C. Colors:
   1. Women: To be selected from Manufacturer's full range of standard colors.
   2. Men: To be selected from Manufacturer's full range of standard colors.

2.04 HARDWARE:

A. Continuous Hinge: Continuous Hinge (57.5") shall be made of Type 304, 14 Gauge Stainless Steel and shall have a Polished Satin Finish. Hinge shall be 3" wide and shall have cam knuckles for gravity type self-closing action. Pivot pin shall be .250" in diameter, and shall be made of Type 304 Stainless Steel. Hinges shall provide emergency access by lifting the door. In-swinging Hinges shall be preset to hold Door open at 15 degrees and Out-swinging Hinges shall be preset to hold Doors in the closed position when unlatched. Hinges shall be pre-drilled for mounting to door and pilaster. Mounting holes shall be at 9" O.C. for mounting to door and pilaster with Theft Proof Stainless Steel Torx Head with Pin Through-Bolts. Brass inserts are unacceptable. Each Hinge is to be labeled by stock number, manufacturer, and left or right hand. Furnish one Hinge per door.

B. Slide Latch: Slide Latch shall be Heavy Duty Type 304 Cast Stainless Steel with a Polished Satin Finish. The Slide Latch shall be surface mounted and shall require less than five (5) lbs to operate. The slide bar shall be .150" thick, 1.020" wide and 3.720" long. Latch shall have an internal Stainless Steel buffering spring to prevent damage when door is inadvertently slammed against the Latch. Mounting holes are to be spaced at 3.50" O.C. Latch knob is to be riveted to the slide bar and then welded to insure that the knob will not come off. The Slide Latch shall be mounted to the Door with Theft Proof Stainless Steel Torx Head with Pin Through-Bolts. Stamped Stainless Steel Slide Latches (with or without plastic or nylon parts) are unacceptable. The stock number shall be molded into the back of the Slide Latch for ease in identification. Furnish one per door.

C. Strike and Keeper: Strike and Keeper shall be Heavy Duty Type 304 Cast Stainless Steel with a Polished Satin Finish. The Strike and Keeper shall provide emergency egress by lifting of the door. The Strike and Keeper shall be 2.50" high, with the mounting holes at 1.50" O.C. The wall thickness shall be a minimum of .125". The Strike and Keeper shall be mounted to the Pilaster with Theft Proof Stainless Steel Torx Head with Pin Through-Bolts. The Strike and Keeper shall have an integral rubber bumper Door Stop. The integral bumper Door Stop shall be rated and able to withstand a sudden impact of 350 lbs. Extra door stops that encumber the door opening and create a hazard are unacceptable. Stamped Stainless Steel Strike and Keepers are unacceptable. The stock number shall be molded into the back of the Strike and Keeper for ease in identification. Furnish one per door.

D. Coat Hook: Coat Hook shall be made of Heavy Duty Type 304 Cast Stainless Steel with a Polished Satin Finish. Coat Hook and Bumper shall be 2.340" high, 1.230" wide and shall protrude out from the door 3.05". The hook portion shall have a finished diameter of .250". Coat Hooks shall be mounted to the Door with Theft Proof Stainless Steel Torx Head with Pin Through-Bolts. Stamped Stainless Steel Coat Hooks are unacceptable. The stock number shall be molded into the back of the Coat Hook and Bumper for ease in identification. Furnish one per door.

E. Pull Handle: Pull Handle shall be made of Heavy Duty Type 304 Cast Stainless Steel with a Polished Satin Finish. Chrome Plated Zamac and Stamped Stainless Steel Door Pulls are unacceptable. Pull Handle shall protrude from the face of the door .940" and shall be 4.735" long. The Pull Handle shall have mounting holes drilled and tapped for 10/24 threads at 3.50" O.C. The Pull Handle shall be .655" wide. The stock number shall be molded into the back of the Pull Handle for ease in identification. Furnish two for each Disabled Accessible door.

F. Door Stop: Door Stop shall be Heavy Duty Type 304 Cast Stainless Steel with a Polished Satin Finish. Chrome Plated Zamac and Stamped Stainless Steel Door Stops are unacceptable. Door Stop shall have a 2.125" base diameter and shall protrude 1.80" from the wall. The bumper at the end of the Door Stop shall be .250" thick. The diameter of the shaft shall be .6875". The stock number shall be molded into the back of the Door Stop for ease in identification. Furnish one for each Disabled Accessible or out-swing door.

G. Continuous Brackets: Brackets shall be Full High (57.5"), Extruded 6063-T5 Aluminum with a Satin Anodized finish. The minimum weight shall be 1.685 pounds per lineal foot. Each Bracket is to have a minimum wall thickness of .125". Continuous Double Ear and Single Ear Brackets shall be used to mount panel/pilaster to wall. Continuous 'U' Brackets shall be used to mount panel to pilaster. Inside opening of Bracket shall be .75" for panels and 1.00" for pilasters. All holes in Brackets for mounting to wall and panel/pilaster shall be pre-drilled. Holes are to be spaced at 9" O.C. along the full length of the Bracket for a total of fourteen (14) holes for mounting to wall, and seven (7) for mounting to panel or pilaster. Panels and Pilasters shall be secured to Brackets with Theft Proof Stainless Steel Torx Head with Pin Through-Bolts. Attachments made with screws into core material are unacceptable.

H. Overhead Bracing (Headrail): Continuous Heavy Duty Extruded 6063-T5 Aluminum Headrail with Anti-Grip profile. Headrail shall have a Satin Anodized finish and shall have integral reinforcing channel and curtain track. Provide Headrail Corner Brackets, Wall Brackets, and Headrail End Caps as required. The Headrail and Aluminum Headrail Corner Brackets shall have a minimum wall height of 2". The minimum wall thickness of the Headrail and Headrail Corner Brackets shall be .125". The Headrail Wall Brackets shall be made of Type 304, 18 Gauge Stainless Steel.

I. Pilaster Shoes: Pilaster Shoes shall be Type 304, 18 Gauge Stainless Steel and shall have a #4 Polished Satin Finish. Shoes shall be 3" high and shall have an integral heel for structural connection to the floor with Stainless Steel fasteners. Shoes shall be attached to Pilasters with Theft Proof Stainless Steel Torx Head with Pin Screws. Zinc Plated fasteners for connection to the floor are unacceptable.

J. Anchorages and Fasteners: All Fasteners shall be Type 304 Stainless Steel and shall have Theft Proof Torx Heads with Pin. Stainless Steel Through-Bolts shall withstand a direct pull force in excess of 2000 lbs. each. All Fasteners shall be Through-Bolted, unless noted otherwise. Chrome Plated Steel, Zinc Plated Steel and Brass Fasteners are not acceptable.

2.05 FABRICATION:

A. General: Provide Doors, Panels, Pilasters and Urinal Screens fabricated for the partition system, complete with all accessories and hardware listed above and as required for installation of a fully functional system, unless otherwise noted. Provide units with cutouts and drilled holes to receive partition-mounted hinges, accessories, and grab bars as indicated.

B. Pilasters: Each Pilaster shall have zinc plated threaded insert(s) threaded into the base of the Pilaster to receive 5/16" X 2.5" Type 304 Stainless Steel Jack Bolt(s) for leveling purposes. Pilaster Shoe shall be 3" high Type 304, 18 Gauge Stainless Steel and shall have an integral heel for structural connection to the floor with Stainless Steel fasteners.

C. Doors: Unless otherwise indicated, provide 24" (610 mm) wide in-swinging doors for standard Toilet Partitions and 36" (914 mm) wide out-swinging doors with a minimum 32" (813 mm) wide clear opening for Partitions indicated to be Handicapped Accessible.

D. Floor Anchored Privacy Screens: Furnish Privacy Screens consisting of a pilaster and a panel of the same construction and finish as the Toilet Partitions. Furnish in accordance with the drawings.

PART 3 - EXECUTION:

3.01 SITE INSPECTION:

A. Verify that room dimensions are in accordance with Toilet Partition Shop Drawings. Inspect walls to insure that they are plumb and suitable for the Wall Brackets.

B. Check location of entrance doors and location of plumbing fixtures. Verify that these are in accordance with Toilet Partition Shop Drawings and that there is adequate clearance between plumbing fixtures and dividing panels or screens.

C. Have any inappropriate conditions corrected before beginning installation.

3.02 INSTALLATION:

A. Comply with manufacturer's written installation instructions. Install Partitions rigid, straight, plumb, and level. Provide clearances of not more than .50" (13 mm) between pilasters and panels, and not more than 1.0" (25 mm) between pilasters/panels and walls. No evidence of drilling, cutting and patching shall be visible in finished work.

B. Overhead-Braced Floor Supported Partitions: Secure Pilasters to floor and level, plumb, and tighten. Maintain proper Door Openings and secure continuous Overhead Brace (Headrail) to each pilaster with not less than two (2) Theft Proof Stainless Steel Torx Head with Pin Through-Bolts. Hang Doors and adjust so that tops of Doors are parallel with Overhead Brace and are the same height as the panels when doors are in closed position.

C. Screens: Attach with anchoring devices according to manufacturer's written instructions and to suit supporting structure. Set units level and plumb.

3.03 ACCESSORIES:

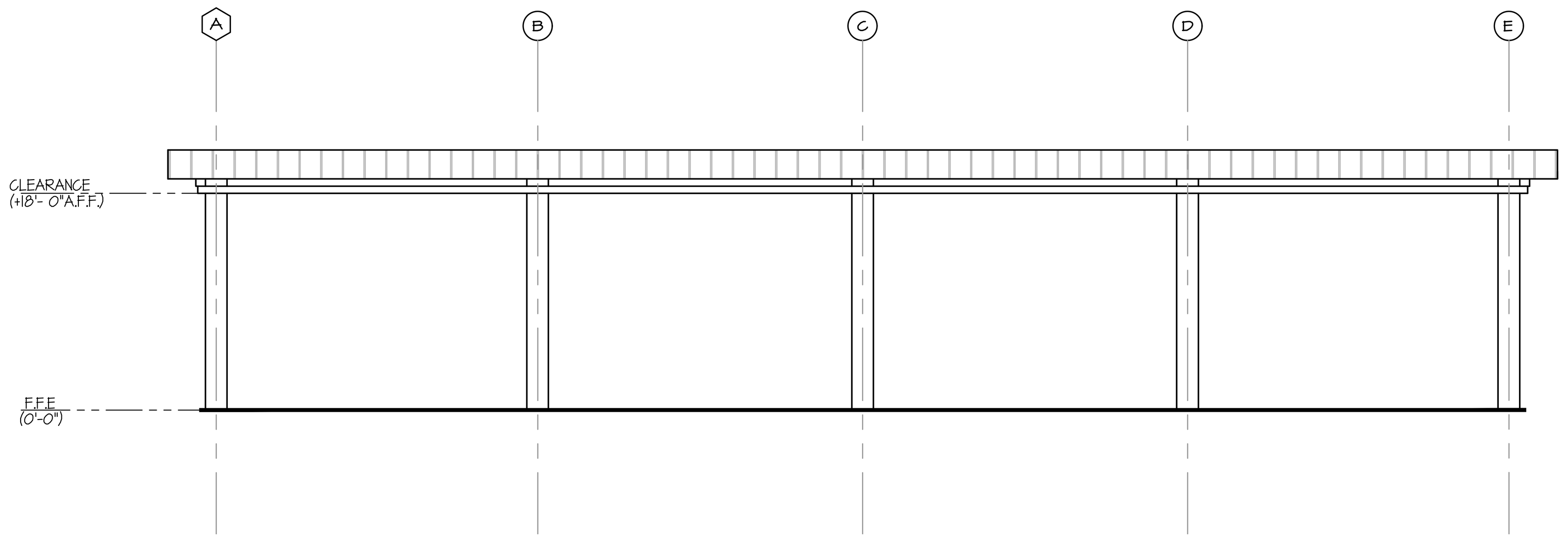
A. Mount accessories to Partition units in accordance with accessory manufacturer's instructions.

3.04 ADJUSTING AND CLEANING:

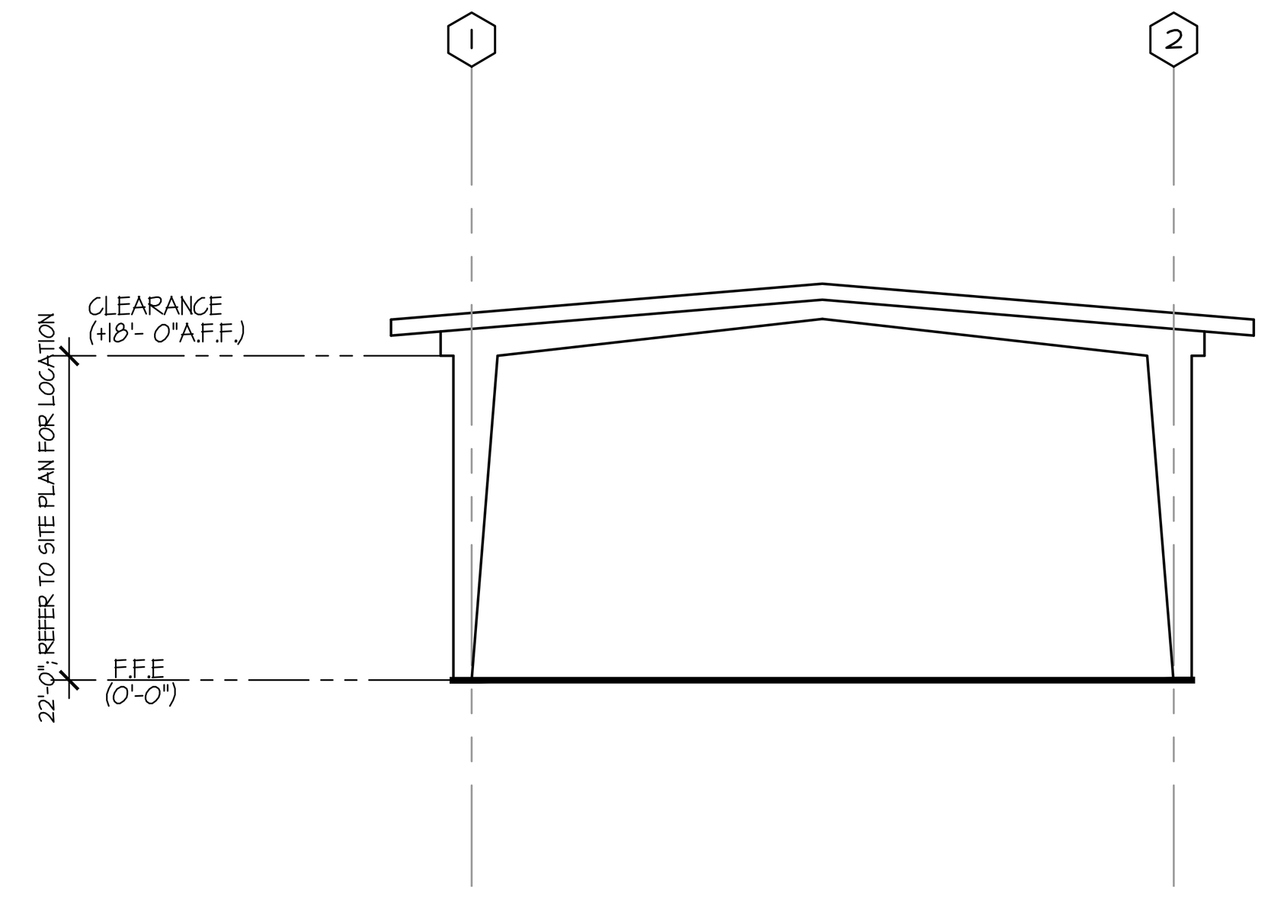
A. Hardware Adjustment: Adjust hardware according to manufacturer's written instructions for proper operation. Adjust cam on all out-swinging doors to hold doors in closed position when unlatched. Adjust cam on all in-swinging doors to hold doors in open position when unlatched.

B. Provide final protection and maintain conditions that ensure Toilet Partitions and Screens are without damage or deterioration at the time of Substantial Completion. Clean all exposed surfaces of Toilet Partitions and hardware.

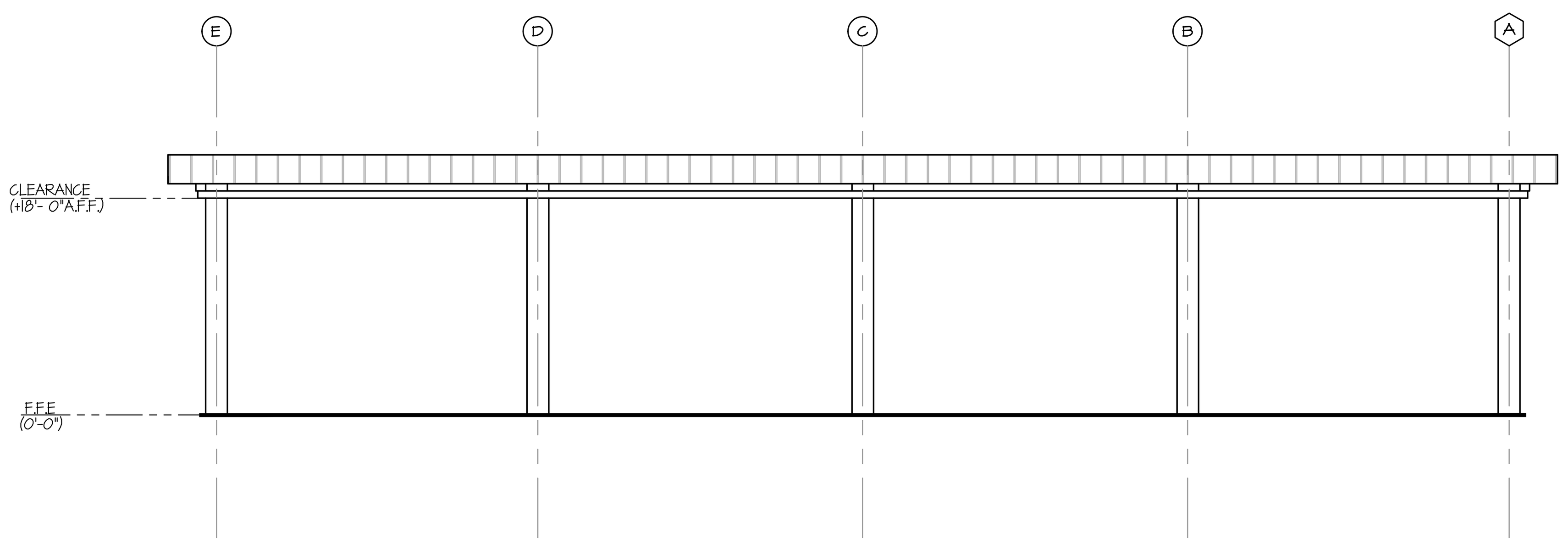
END OF SECTION 10 2113



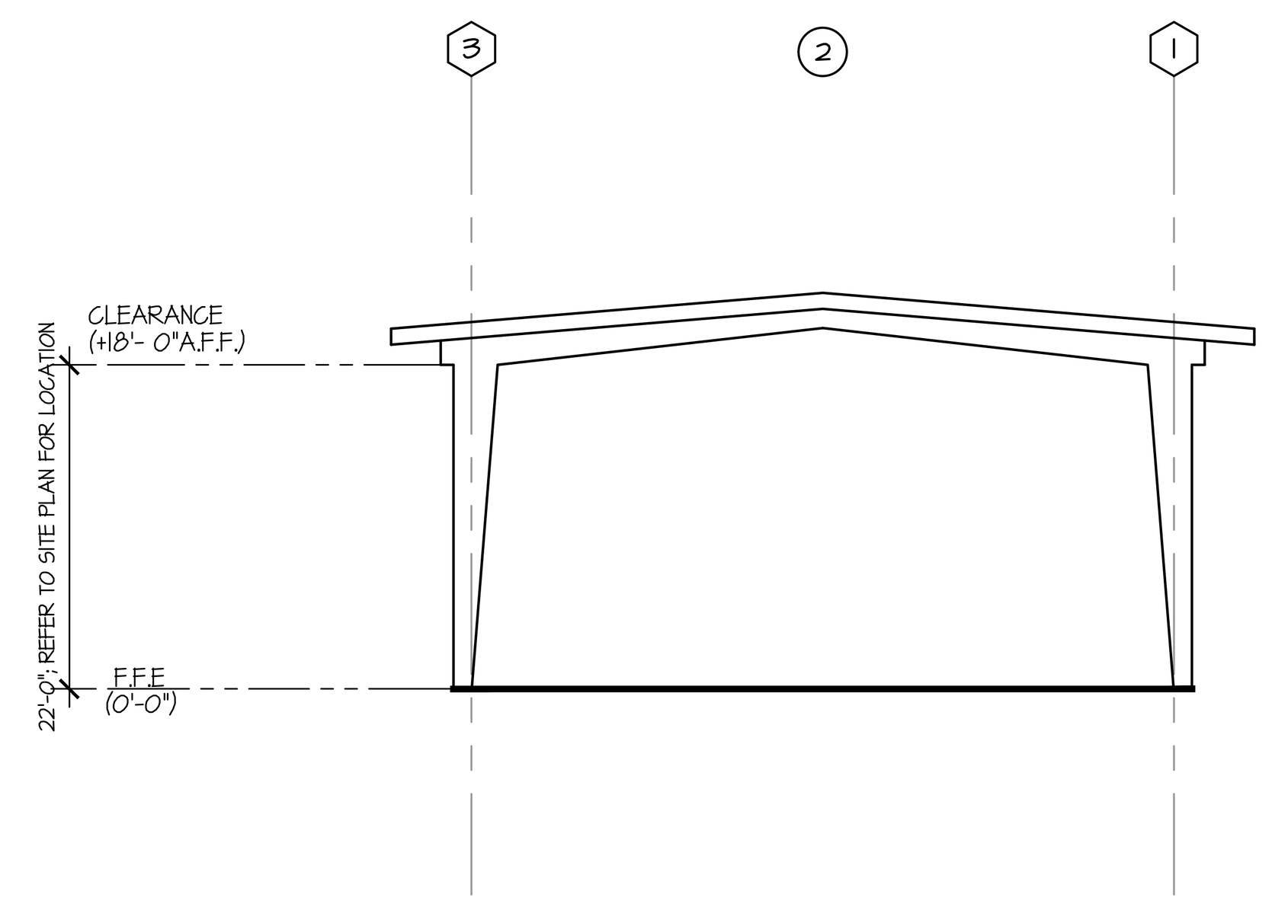
1 A4.01 COVERED EQUIPMENT STORAGE FRONT ELEVATION
SCALE: 3/32" = 1'-0"



2 A4.01 COVERED EQUIPMENT STORAGE LEFT ELEVATION
SCALE: 3/32" = 1'-0"



3 A4.01 COVERED EQUIPMENT STORAGE REAR ELEVATION
SCALE: 3/32" = 1'-0"



4 A4.01 COVERED EQUIPMENT STORAGE RIGHT ELEVATION
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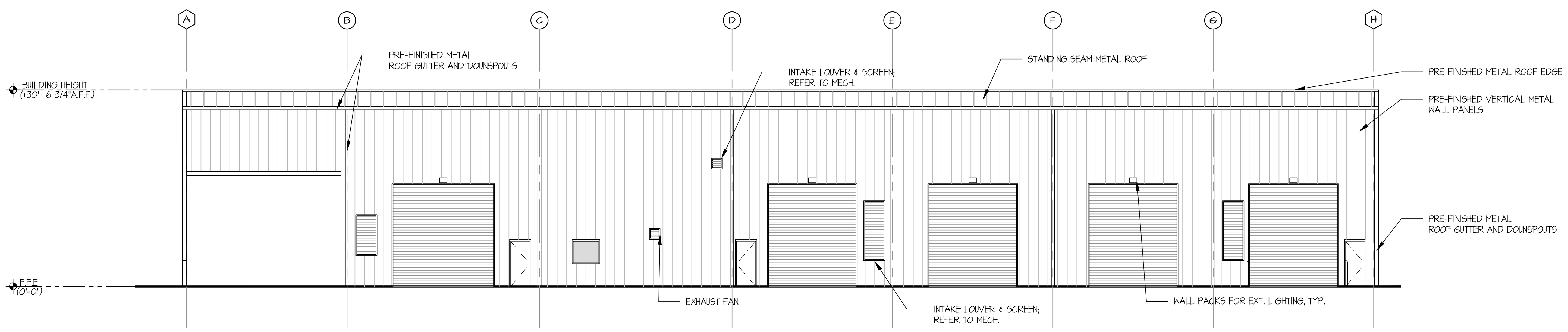

5 A4.01 VEHICLE MAINTENANCE AND STORAGE FRONT ELEVATION
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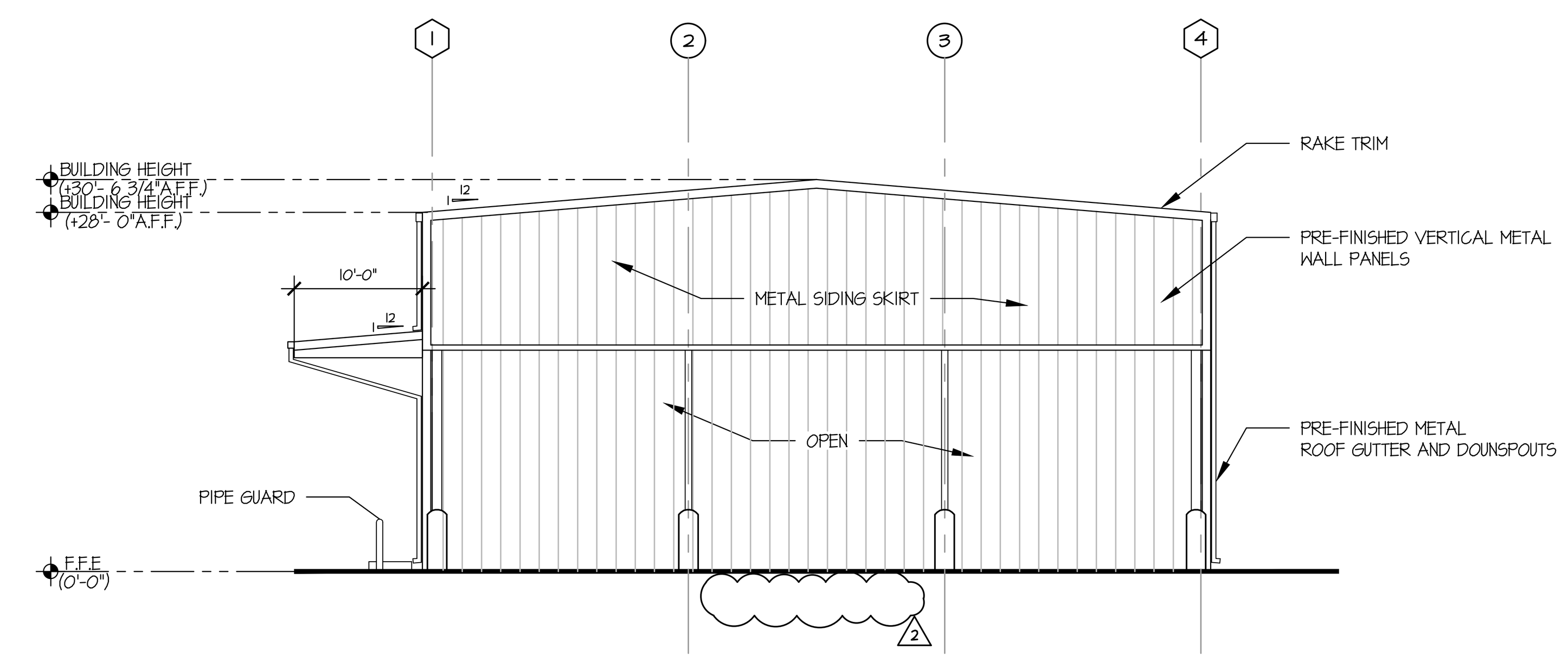

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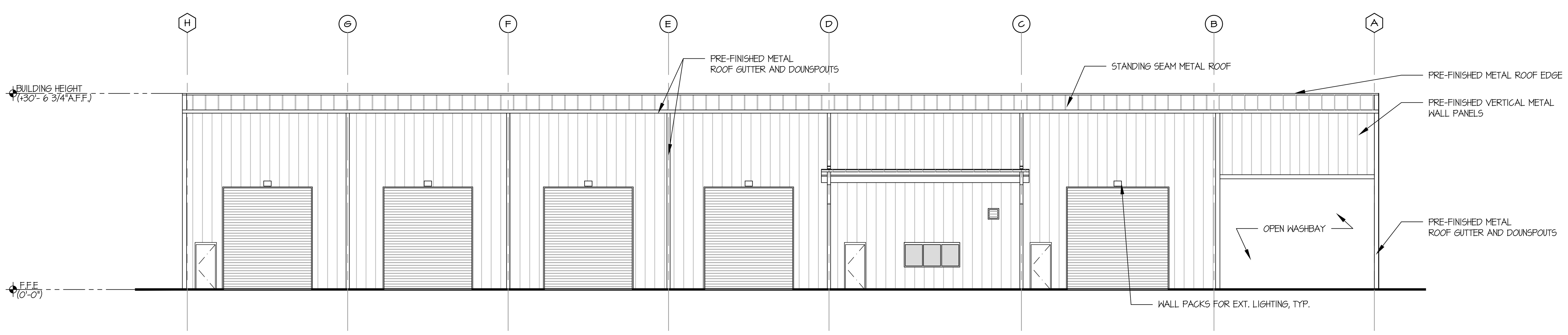

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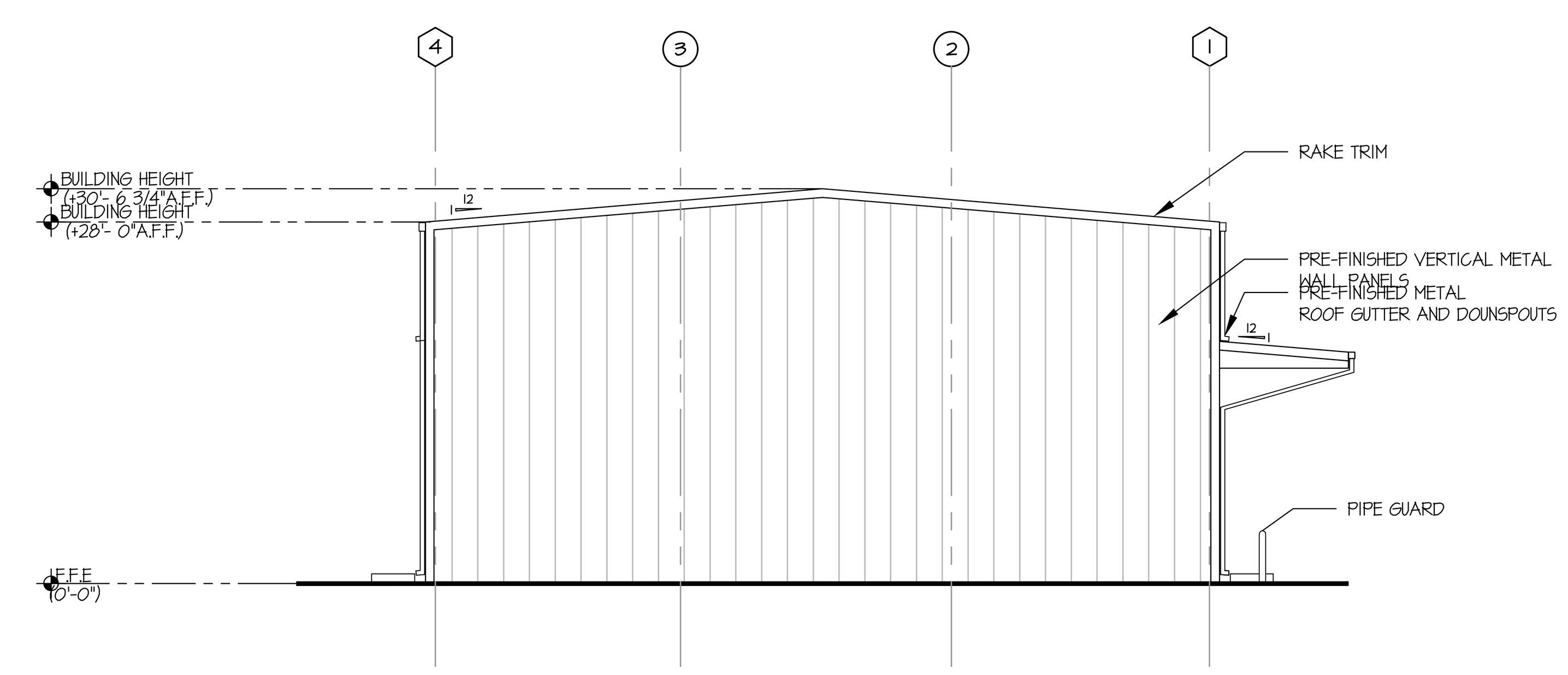

8 A4.01 VEHICLE MAINTENANCE AND STORAGE RIGHT ELEVATION
SCALE: 3/32" = 1'-0"

PROJECT NUMBER
2512
DATE
03/12/26
DRAWN BY
NES,MJM
APPROVED BY

REVISIONS
2 4/15/26 ADDENDUM #2

CITY OF SOCIAL CIRCLE
PUBLIC WORKS DEPARTMENT
New Fleet Facility
New Vine Circle, Social Circle, GA 30025

TURNIPSEED ENGINEERS
ATLANTA AUGUSTA ST. SIMONS ISLAND

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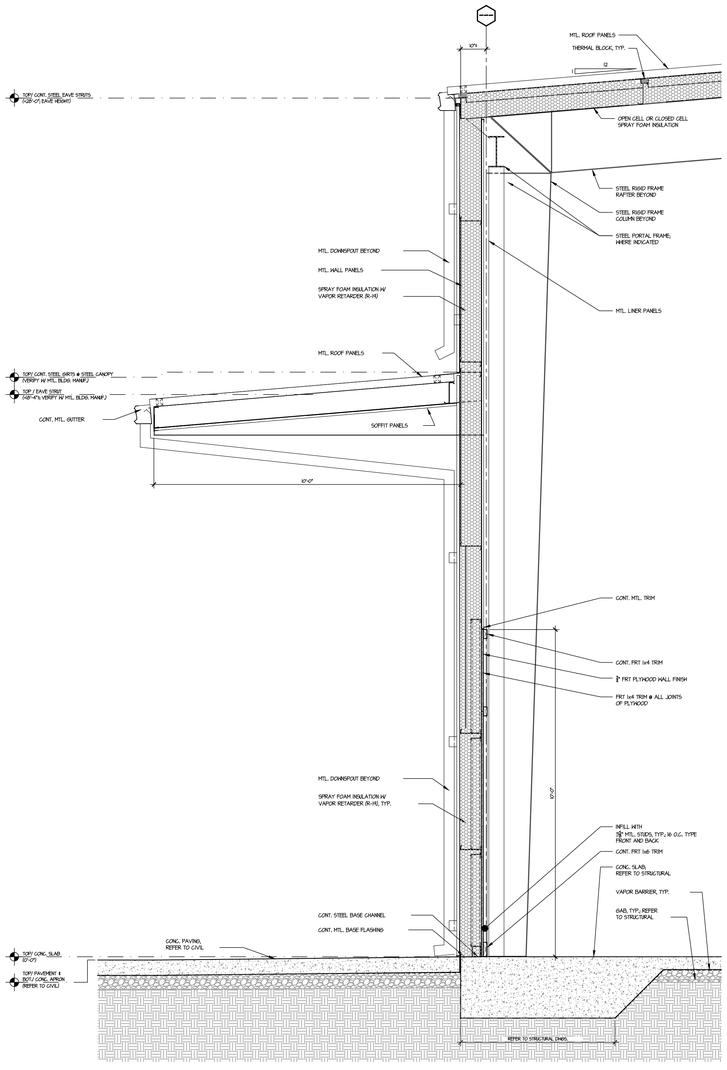
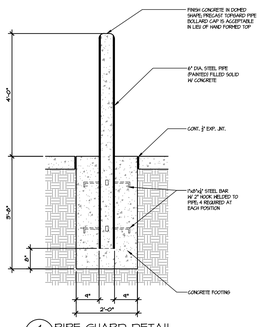
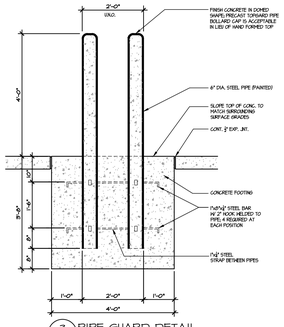
STATE OF GEORGIA
MATTHEW J. MILLARD
REGISTERED ARCHITECT

SHEET NUMBER
A4.01





4 PIPE GUARD DETAIL
A5.01 SCALE: 1/2" = 1'-0"



3 PIPE GUARD DETAIL
A5.01 SCALE: 1/2" = 1'-0"

2 VEHICLE MAINTENANCE WALL SECTION
A5.01 SCALE: 1/2" = 1'-0"

1 VEHICLE MAINTENANCE WALL SECTION
A5.01 SCALE: 1/2" = 1'-0"

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STATE OF GEORGIA
MATTHEW J. MILLARD
CERTIFICATE NO. 018191
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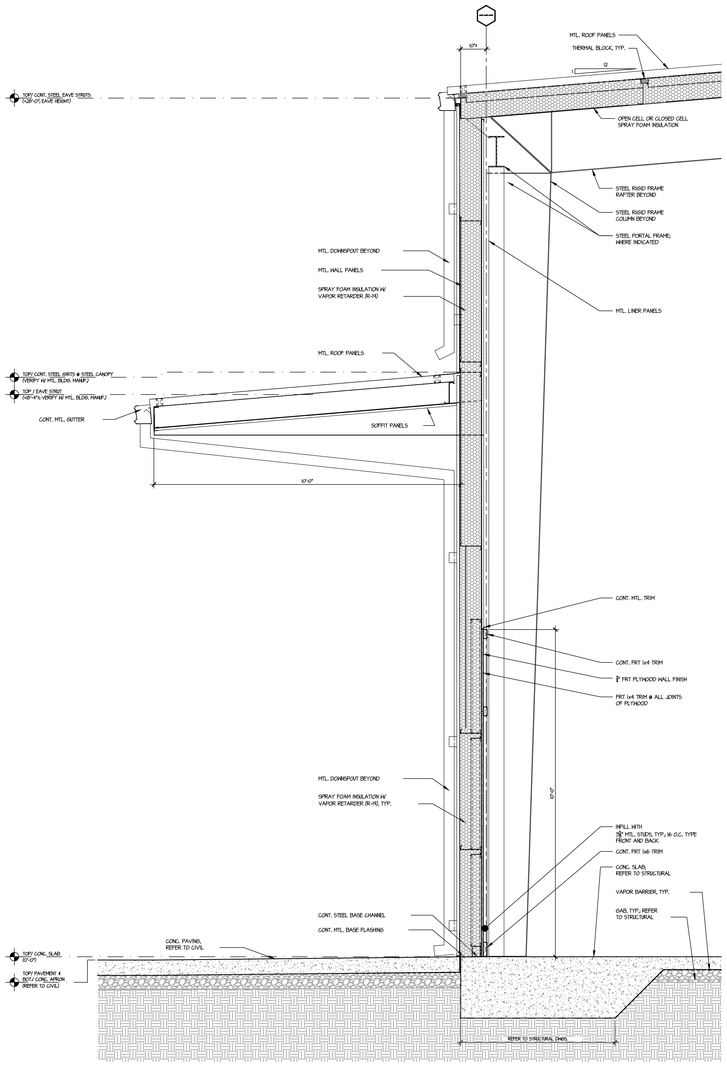
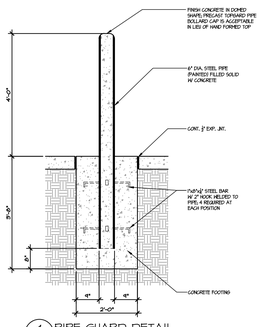
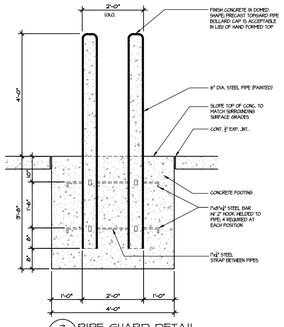




4 PIPE GUARD DETAIL
A5.01 SCALE: 1/2" = 1'-0"



3 PIPE GUARD DETAIL
A5.01 SCALE: 1/2" = 1'-0"

2 VEHICLE MAINTENANCE WALL SECTION
A5.01 SCALE: 1/2" = 1'-0"

1 VEHICLE MAINTENANCE WALL SECTION
A5.01 SCALE: 1/2" = 1'-0"

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MATTHEW J. MILLARD
CERTIFICATE NO. 018191
REGISTERED ARCHITECT

SHEET NUMBER
A5.01

GENERAL NOTES – STRUCTURAL:

CODE: 2024 INTERNATIONAL BUILDING CODE (IBC 2024) WITH GEORGIA AMENDMENTS

DESIGN LOADS

SLAB ON GRADE: LIVE LOAD = 100 PSF

MEZZANINE:
LIVE LOAD = 250 PSF
DEAD LOAD = 60 PSF

STAIRS:
DEAD LOAD = 50 PSF
LIVE LOAD = 100 PSF
PT LOAD OF 300 LB OVER 4 IN^2

ROOF:
LIVE LOAD = 20 PSF
DEAD LOAD = ACTUAL DEAD LOAD OF STRUCTURE +5 PSF MISC.

SNOW:
GROUND SNOW LOAD (Pg) = 5 PSF
EXPOSURE FACTOR = 1.0
THERMAL FACTOR = 1.2
IMPORTANCE FACTOR = 1.10
FLAT ROOF SNOW LOAD = NA
MINIMUM ROOF SNOW LOAD = 5.5

WIND:
ULTIMATE WIND SPEED = 114 MPH
NOMINAL WIND SPEED = 90 MPH
RISK CATEGORY III
EXPOSURE C
IMPORTANCE FACTOR: 1.0
BUILDING CLASSIFICATION :
VEHICLE MAINT BUILDING: PARTIALLY ENCLOSED
TRUCK SHEDS: OPEN
INTERNAL PRESSURE COEFFICIENT:
VEHICLE MAINT BUILDING: GC_{PI} = ±0.55
TRUCK SHEDS: GC_{PI} = 0.00

TORNADO:
EFFECTIVE PLAN AREA A_e = 11,500 SQ FT
TORNADO WIND SPEED V_T = 50 MPH
(DESIGN FOR TORNADO LOADS NOT REQUIRED PER ASCE 7-22 SECTION 32.5.2.)

SEISMIC:
RISK CATEGORY III
SEISMIC IMPORTANCE FACTOR = 1.25
SOIL SITE CLASS D
SHORT PERIOD RESPONSE COEFF S_{DS} = 0.199
1-SEC PERIOD RESPONSE COEFF S_{D1} = 0.132
SEISMIC DESIGN CATEGORY B

ANALYSIS PROCEDURE: BY METAL BUILDING ENGINEER
SEISMIC FORCE RESISTING SYSTEM: BY METAL BUILDING ENGINEER
RESPONSE MODIFICATION FACTOR (R)= BY METAL BUILDING ENGINEER
DEFLECTION AMPLIFICATION FACTOR (Cd)= BY METAL BUILDING ENGINEER.
SEISMIC RESPONSE COEFFICIENT (Cs) = BY METAL BUILDING ENGINEER
BASE SHEAR = BY METAL BUILDING ENGINEER

GENERAL NOTES

1. THE STRUCTURAL DESIGN AND CONSTRUCTION IS BASED ON AND SHALL BE IN ACCORDANCE WITH THE 2018 INTERNATIONAL BUILDING CODE WITH GEORGIA STATE AMENDMENTS.
2. THE CONTRACTOR SHALL VERIFY ALL EXISTING SITE CONDITIONS AND DIMENSIONS ON ARCHITECTURAL AND STRUCTURAL DRAWINGS. CONTRACTOR SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES BETWEEN SITE CONDITIONS, ARCHITECTURAL DRAWINGS, AND STRUCTURAL DRAWINGS PRIOR TO BEGINNING WORK.
3. THE CONTRACTOR SHALL LOCATE ANY AND ALL EXISTING UTILITY LINES AND SHALL COORDINATE WITH ARCHITECTURAL AND STRUCTURAL DETAILS. CONTRACTOR SHALL NOTIFY THE ARCHITECT AND OWNER IMMEDIATELY OF ANY CONFLICTS WITH EXISTING UTILITY LINES.
4. THE STRUCTURAL DRAWINGS REPRESENT THE FINISHED STRUCTURE, UNLESS OTHERWISE INDICATED. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL ASSUME SOLE RESPONSIBILITY FOR ALL MEANS AND METHODS OF CONSTRUCTION AND SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKMEN, OR OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO: BRACING, SHORING FOR THE STRUCTURE AND FOR CONSTRUCTION EQUIPMENT, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUPPORT, AND BRACING FOR CRANES, ETC.
5. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, THAT DETAIL SHALL APPLY FOR ALL SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
6. ALL DIMENSIONS TO TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, SECTIONS, AND DETAILS.
7. SPECIFIC NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
8. ALL SITE PREPARATION AND GRADING, INCLUDING PLACEMENT AND COMPACTION OF FILL AND COMPACTION OF SLAB SUB-GRADE SHALL BE IN ACCORDANCE WITH RECOMMENDATIONS CONTAINED IN THE GEOTECHNICAL INVESTIGATION REPORT REFERENCED IN FOUNDATIONS GENERAL NOTE #1 BELOW.
9. SPECIAL INSPECTIONS SHALL BE CONDUCTED AS REQUIRED BY CHAPTER 17 IN IBC 2018. SEE SPECS.

FOUNDATIONS:

1. FOUNDATION DESIGN IS BASED ON THE PRESUMPTIVE CRITERIA SET FORTH IN THE 2018 INTERNATIONAL BUILDING CODE (IBC 2018) SECTION 1806:

   FOUNDATION SYSTEM: SHALLOW FOOTINGS BEARING ON RESIDUAL SOIL / STRUCTURAL FILL

   ALLOWABLE VERTICAL BEARING PRESSURE: 1,500 PSF (IBC TABLE 1806.2)
2. ACTUAL ALLOWABLE BEARING PRESSURES SHALL BE VERIFIED PRIOR TO FOOTING PLACEMENT BY A GEOTECHNICAL ENGINEER REGISTERED IN THE PROJECT STATE.
3. CONTRACTOR TO PROVIDE FOR DE-WATERING IN EXCAVATIONS FROM EITHER SURFACE WATER, GROUND WATER, OR SEEPAGE.
4. CONTRACTOR SHALL PROVIDE AND INSTALL ALL CRIBBING, SHEATHING AND SHORING REQUIRED TO SAFELY RETAIN THE EARTH BANKS.
5. CONTRACTOR SHALL PROTECT ALL UTILITY LINES, ETC., ENCOUNTERED DURING EXCAVATION AND BACKFILLING.
6. ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED, BUT NOT BEFORE CONCRETE HAS ATTAINED FULL DESIGN STRENGTH.
7. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COMPLY WITH THE REQUIREMENTS AND RECOMMENDATIONS OF A REGISTERED GEOTECHNICAL ENGINEER IN ORDER TO ACHIEVE THE MINIMUM ALLOWABLE BEARING PRESSURE NOTED ABOVE. ALL SITE PREPARATION AND GRADING, INCLUDING MOISTURE CONDITIONING, PLACEMENT, AND COMPACTION OF FILL AND COMPACTION OF SLAB SUB-GRADE SHALL BE IN ACCORDANCE WITH RECOMMENDATIONS BY THE GEOTECHNICAL ENGINEER.

CONCRETE NOTES

1. ALL CONCRETE DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318 AND ACI 301.
2. PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I, UNLESS NOTED OTHERWISE.
3. THE CONTRACTOR SHALL HAVE THE CONCRETE TEST REPORTS AVAILABLE AT THE JOB SITE AT ALL TIMES.
4. CONCRETE SLUMP SHALL BE 3" TO 5" AT TIME OF PLACEMENT.
5. CONCRETE MIX DESIGNS SHALL BE ESTABLISHED BY THE SUPPLIER IN ACCORDANCE WITH THE ABOVE REFERENCED STANDARDS. MIX DESIGNS SHALL BE SUBMITTED WITH BACK-UP DATA PER ACI 318 TO THE ARCHITECT FOR REVIEW PRIOR TO CONCRETE PLACEMENT.
6. ALL BELOW-FLOOR PIPING AND CONDUIT SHALL BE PLACED BELOW THE SLAB AND NOT WITHIN THE SLAB. VERTICAL PENETRATIONS THROUGH THE SLAB ARE ACCEPTABLE.
7. THE FOLLOWING CHART SHALL BE USED TO DETERMINE MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, MAXIMUM WATER TO CEMENTITIOUS MATERIAL RATIO (W/CM), AND ENTRAINED AIR CONTENT UNLESS SPECIFICALLY NOTED OTHERWISE:

	EXPOSURE CATEGORY						
STRUCTURAL COMPONENT	F^1 CLASS	$S^{5,7}$ CLASS	P CLASS	C^6 CLASS	MIN^2 F'c (PSI)	MAX^3 W/ CM	$ENTRAINED^4$ AIR CONTENT ±1½%
FOOTINGS	F0	S0	P0	C0	3000	N/A	N/A
SLABS ON GRADE	F1	S0	P0	C0	4500	0.45	5%
ELEVATED COMPOSITE SLAB	F1	S0	P0	C0	4500	0.45	5%

NOTES:
1. FOR F3 CLASS SEE ACI 318 TABLE 26.4.2.2(b) FOR ADDITIONAL CONCRETE MIXTURE REQUIREMENTS.
2. MINIMUM IS BASED ON MAXIMUM OF DESIGN AND GOVERNING EXPOSURE CLASS REQUIREMENTS.
3. MAXIMUM IS BASED ON GOVERNING EXPOSURE CLASS REQUIREMENTS.
4. BASED ON MAXIMUM ¾" AGGREGATE SIZE.
5. SEE ACI 318 TABLE 19.3.2.1 FOR CEMENTITIOUS MATERIAL AND CALCIUM CHLORIDE ADMIXTURE RESTRICTIONS.
6. SEE ACI 318 TABLE 19.3.2.1 FOR MAXIMUM WATER-SOLUBLE CHLORIDE ION CONTENT.
7. SO EXPOSURE ASSUMED. GC SHALL VERIFY WITH GEOTECHNICAL REPORT.

REINFORCING STEEL NOTES

1. CONCRETE CLEAR COVER AT REINFORCING STEEL SHALL BE (UNLESS NOTED OTHERWISE):

CONCRETE CAST AGAINST EARTH:	3"
CONCRETE EXPOSED TO EARTH OR WEATHER:	2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
#11 AND SMALLER IN SLABS, WALLS, AND JOISTS:	¾"
BEAM AND COLUMN TIES AND STIRRUPS:	1½"

2. REINFORCING STEEL BARS SHALL HAVE A YIELD STRENGTH OF 60 KSI AND SHALL CONFORM TO ASTM A615.
3. REINFORCING BARS TO BE WELDED SHALL CONFORM TO ASTM A706.
4. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. WHERE WWF SHEETS MEET THE SHEETS SHALL BE LAPPED 2 FULL SQUARES AND TIED.
5. REINFORCING BAR LENGTHS NOTED ON STRUCTURAL DRAWINGS SHALL BE IN ADDITION TO THE LENGTH OF ANY HOOKS.
6. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.
7. WALL FOOTING AND SLAB TURNDOWN REINFORCEMENT SHALL BE CONTINUOUS AT COLUMNS.
8. CONTRACTOR SHALL PROVIDE CORNER BARS AT ALL CORNERS.
9. REINFORCING STEEL MARKED "CONTINUOUS" SHALL BE LAPPED W/ CLASS "B" LAP SPLICE UNLESS SPECIFICALLY DETAILED OTHERWISE.
10. LAPS SPLICES SHALL BE CLASS "B" UNLESS NOTED OTHERWISE, AND SHALL CONFORM TO ACI 318.
11. MINIMUM LAPS OF CONCRETE REINFORCING BARS SHALL BE AS FOLLOWS:

#4 BARS	2'-4"
#5 BARS	3'-0"
#6 BARS	3'-7"

12. CONTRACTOR SHALL NOT PLACE ANY REINFORCING UNTIL APPROVED SHOP DRAWINGS ARE RECEIVED ON THE JOB SITE.

STRUCTURAL STEEL NOTES

1. DETAILING, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE AISC 360 SPECIFICATION FOR STEEL BUILDINGS AND THE AISC STEEL CONSTRUCTION MANUAL.
2. STRUCTURAL STEEL FRAMING AND ERECTION SHALL COMPLY WITH THE LATEST OSHA STEEL ERECTION STANDARDS.
3. STRUCTURAL STEEL SHAPES SHALL BE OF THE FOLLOWING YIELD STRENGTHS AND ASTM SPECIFICATIONS:

WF COLUMNS AND BEAMS:	50KSI	ASTM A992
HSS TUBES	46KSI	ASTM A500 GRADE B
PLATES, ANGLES, AND CHANNELS:	36KSI	ASTM A36
PIPE	35KSI	ASTM A53 GRADE B

4. ALL CONNECTIONS SHALL BE SHOP WELDED AND FIELD BOLTED UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS.
5. BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RCSC-2000 SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 AND ASTM A490 BOLTS.
6. ALL FIELD WELDING SHALL BE DONE WITH E-70XX ELECTRODES.
7. WELDED CONNECTIONS SHALL CONFORM TO AWS D1.1 STRUCTURAL WELDING CODE BY THE AMERICAN WELDING SOCIETY, LATEST EDITION.
8. PROOF OF CERTIFICATION OF ALL WELDERS PERFORMING FIELD WELDING SHALL BE AVAILABLE AT THE JOB SITE AT ALL TIMES.
9. FIELD WELDS SHALL BE CLEARED OF ALL SPOILS AND RE-PRIMED.
10. THE GENERAL CONTRACTOR SHALL SUBMIT TO THE ARCHITECT, FOR REVIEW, ENGINEERED AND CONTRACTOR-APPROVED SHOP DRAWINGS SHOWING SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS, AND ERECTION DRAWINGS FOR ALL STRUCTURAL STEEL.
11. ALL CONNECTIONS SHALL BE DESIGNED BY A CONNECTION ENGINEER EMPLOYED BY OR CONTRACTED TO THE FABRICATOR. THE CONNECTION ENGINEER SHALL BE A LICENSED PROFESSIONAL ENGINEER REGISTERED IN THE PROJECT STATE. CONNECTION DETAILING SHALL BE PERFORMED BY A DETAILER UNDER THE RESPONSIBLE CHARGE OF THE CONNECTION ENGINEER. CONNECTION DESIGN & DETAILING SHALL BE PERFORMED USING RATIONAL ENGINEERING DESIGN AND STANDARD PRACTICE IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. THE GENERAL DETAILS SHOWN ON THE DRAWINGS ARE CONCEPTUAL ONLY AND DO NOT INDICATE THE REQUIRED NUMBER OF BOLTS OR WELD SIZES, UNLESS SPECIFICALLY NOTED. CONNECTION CALCULATIONS, SIGNED & SEALED BY THE CONNECTION ENGINEER, SHALL BE SUBMITTED WITH THE STRUCTURAL STEEL SHOP DRAWINGS FOR THE ENGINEER'S RECORD.
12. NON-COMPOSITE BEAM CONNECTIONS SHALL BE DESIGNED BY THE FABRICATOR'S CONNECTION ENGINEER FOR THE REACTION DUE TO THE MAXIMUM ALLOWABLE LOAD FOR THE APPROPRIATE SPAN AND SHAPE BASED ON THE ALLOWABLE UNIFORM LOAD TABLES IN THE ABOVE REFERENCED STANDARD.
13. COMPOSITE BEAM CONNECTIONS SHALL BE DESIGNED FOR THE REACTIONS INDICATED IN THE COMPOSITE STEEL FRAMING NOTES ON THIS SHEET, UNLESS NOTED OTHERWISE IN THESE STRUCTURAL DRAWINGS.
14. MINIMUM NUMBER OF BOLT ROWS BASED ON MEMBER DEPTH FOR W & C SHAPES ARE AS NOTED BELOW. ADDITIONAL BOLT ROWS MAY BE REQUIRED AS REQUIRED BY CONNECTION ENGINEER'S DESIGN PER NOTES ABOVE.

UP TO 12" DEEP..............................	2 ROWS
14" TO 16" DEEP............................	3 ROWS
18" TO 21" DEEP............................	4 ROWS
24" DEEP..	5 ROWS

15. ALL SIMPLE SHEAR CONNECTIONS SHALL BE CAPABLE OF END ROTATION AS PER THE REQUIREMENTS IN SECTION J1.2 OF THE ABOVE REFERENCED STANDARD FOR UNRESTRAINED MEMBERS.
16. AFTER FABRICATION, ALL STEEL SHALL BE CLEANED OF ALL RUST, LOOSE MILL SCALE AND OTHER FOREIGN MATERIALS AND SHOP PAINTED WITH FABRICATOR'S STANDARD RUST-INHIBITING PRIMER TO PROVIDE A MINIMUM DRY FILM THICKNESS OF 3 MILS. SHOP PRIMER SHALL BE BLOCKED OUT ON SURFACES THAT RECEIVE FIELD WELDS, SURFACES THAT RECEIVE FIELD-WELDED HEADED STUDS, SURFACES THAT RECEIVE SPRAY-ON FIREPROOFING AND SURFACES AT SLIP-CRITICAL BOLTS.
17. UNLESS NOTED AS GALVANIZED ON THE DRAWINGS, ALL STRUCTURAL STEEL EXPOSED TO THE WEATHER SHALL BE SHOP PRIMED, CLEANED & PAINTED AS FOLLOWS: SSPC-SP6, 2-PACK EPOXY POLYAMIDE ZINCH-RICH PAINT WITH HIGH-BUILD EPOXY TOPCOAT.
18. ALL STEEL EXPOSED TO EARTH SHALL BE PAINTED WITH A BITUMINOUS COATING.
19. FIELD TOUCH-UP PAINTING: ALL STEEL MEMBERS AND THEIR CONNECTIONS, THAT ARE EXPOSED TO VIEW, SHALL BE TOUCHED-UP AT FIELD WELDS, AT AREAS WHERE SHOP PRIMER WAS BLOCKED OUT AT SLIP-CRITICAL BOLTS AND AT AREAS THAT HAVE BEEN SCRATCHED OR SCRAPED DURING ERECTION. TOUCH-UP PAINT COLOR SHALL MATCH EXISTING. TOUCH-UP PAINTING SHALL BE COMPLETED BY THE FABRICATOR PRIOR TO FABRICATOR'S DEMOBILIZATION FROM THE JOB SITE. TOUCH-UP AT GALVANIZED COMPONENTS SHALL UTILIZE A ZINC-RICH PAINT.
20. THERE SHALL BE NO FIELD CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT THE PRIOR APPROVAL OF THE ENGINEER.

COMPOSITE STEEL FRAMING:

1. COMPOSITE BEAM CONNECTIONS SHALL BE DESIGNED FOR THE REACTIONS SHOWN BELOW OR PER STRUCTURAL STEEL NOTE NO. 12, WHICHEVER IS GREATER. UNDER NO CIRCUMASTANCES SHALL THE NUMBER OF BOLT ROWS BE LESS THAN THAT INDICATED IN STRUCTURAL STEEL NOTE 14:

W8, W10	15 KIPS
W12, W14	30 KIPS
W16, W18	55 KIPS
W21	70 KIPS
W24..	80 KIPS

2. HEADED STUD SHEAR CONNECTORS SHALL COMPLY WITH ASTM A108 GRADES 1016 THROUGH 1020. WELDING AND TESTING SHALL CONFORM TO AWS D1.1-85 STUD WELDING.
3. DECK MANUFACTURER SHALL SUBMIT LAYOUT OF SHEAR STUD QUANTITIES AND ARRANGEMENT FOR EVERY BEAM TO STRUCTURAL ENGINEER FOR APPROVAL.
4. TOP FLANGE OF STRUCTURAL STEEL BEAMS TO RECEIVE STUDS SHALL BE FREE OF PAINT, SCALE, RUST AND OTHER SUBSTANCES WHICH WOULD BE DETRIMENTAL TO THE WELDING OF STUDS THRU DECK.
5. SCREEDS SHALL BE SET TO ASSURE A LEVEL FLOOR SLAB. SCREEDS SHALL BE ADJUSTABLE SO THAT WHEN THE STRUCTURAL SYSTEM DEFLECTS WITH THE ADDITION OF WET CONCRETE, THE FLOOR WILL BE LEVEL BETWEEN MAJOR SUPPORT MEMBERS. THE COST FOR ADDITIONAL CONCRETE DUE TO DEFLECTION OF THE STRUCTURAL SYSTEM SHALL BE BORNE BY THE CONTRACTOR. ACTUAL THICKNESS OF CONCRETE ABOVE THE METAL DECK SHALL NOT BE LESS THAN THE THICKNESS INDICATED ON THE DWGS.
6. METAL DECK SHALL CONFORM TO THE STEEL DECK INSTITUTE (SDI) SPECIFICATIONS, LATEST EDITION.
7. ATTACH COMPOSITE METAL DECK TO ALL SUPPORTING MEMBERS WITH ⅝"Φ PUDDLE WELDS @ 12" O.C. EACH HEADED STUD WELDED THRU THE DECK TO THE SUPPORTING MEMBER MAY BE CONSIDERED AS REPLACING ONE PUDDLE WELD. PROVIDE (3) #10 SIDE LAP SCREWS PER SPAN BUT IN NO CASE SHALL SIDE LAPS BE SPACED MORE THAN 36" O.C.
8. COMPOSITE DECK SHALL BE PROVIDED WITH ADEQUATE LENGTH TO ALLOW 3 SPAN OR MORE INSTALLATION. DECK SECTION PROPERTIES SHALL BE SUFFICIENT TO ACHIEVE THE UNSHORED CLEAR SPANS REPRESENTED ON THE DRAWINGS.

STEEL STAIRS, GUARD RAILS & GRAB BARS NOTES:

1. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND EXACT CONFIGURATION OF STAIRS. DO NOT SCALE OFF STRUCTURAL DRAWINGS FOR DIMENSIONS.
2. UNLESS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS, ALL STAIRS, LANDINGS, LANDING POSTS, GUARD RAILS, HAND RAILS AND THEIR CONNECTIONS TO THE SUPPORTING STRUCTURE SHALL BE DESIGNED BY THE STAIR SUPPLIER/FABRICATOR TO SUPPORT THE DESIGN LOADS PRESCRIBED IN THE GOVERNING BUILDING CODE AND AS NOTED BELOW.
3. HANDRAIL ASSEMBLIES AND GUARDS SHALL BE DESIGNED TO RESIST A LOAD OF 50 PLF APPLIED IN ANY DIRECTION AT THE TOP AND TO TRANSFER THIS LOAD THROUGH THE SUPPORTS TO THE STRUCTURE.
4. HANDRAIL ASSEMBLIES AND GUARDS SHALL BE ABLE TO RESIST A SINGLE CONCENTRATED LOAD OF 200 LBS APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE TOP, AND SHALL HAVE ATTACHMENT DEVICES AND SUPPORTING STRUCTURE TO TRANSFER THIS LOADING TO APPROPRIATE STRUCTURAL ELEMENTS OF THE BUILDING. THIS LOAD NEED NOT BE ASSUMED TO ACT CONCURRENTLY WITH THE LOADS SPECIFIED IN THE PRECEDING NOTE.
5. INTERMEDIATE RAILS (ALL THOSE EXCEPT THE HANDRAIL), BALUSTERS AND PANEL FILLERS SHALL BE DESIGNED TO WITHSTAND A HORIZONTALLY APPLIED NORMAL LOAD OF 50 LBS ON AN AREA EQUAL TO 1 SQUARE FOOT, INCLUDING OPENINGS AND SPACE BETWEEN RAILS. REACTIONS DUE TO THIS LOADING ARE NOT REQUIRED TO BE SUPERIMPOSED WITH REACTIONS DUE TO LOADS SPECIFIED IN THE PRECEDING NOTES.
6. GRAB BARS SHALL BE DESIGNED TO RESIST A SINGLE CONCENTRATED LOAD OF 250 LBS APPLIED IN ANY DIRECTION AT ANY POINT.
7. MEMBERS SHALL BE ALIGNED AND SHALL BE FIT TOGETHER WITHOUT VISIBLE FLAWS, GAPS, OR OFFSETS.
8. WELD CORNERS AND SEAMS CONTINUOUSLY AND IN ACCORDANCE WITH AWS RECOMMENDATIONS. GRIND ALL EXPOSED WELDS SMOOTH AND FLUSH TO MATCH AND BLEND WITH ADJOINING SURFACES.
9. CLEAN ARCHITECTURALLY EXPOSED MEMBERS PER SSPC_SP-10-63 AND PRIME WITH A ZINC-RICH PRIMER.
10. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR OTHER DETAILS INCLUDING (BUT NOT LIMITED TO) STEEL FINISHES AND RAILINGS.

ARCHITECTURALLY EXPOSED STEEL NOTES

1. ALL ARCHITECTURALLY EXPOSED STEEL SHALL COMPLY WITH AISC SPECIFICATION FOR ARCHITECTURALLY EXPOSED STRUCTURAL STEEL.
2. WELD CORNERS AND SEAMS, AND GRIND EXPOSED WELDS SMOOTH AND FLUSH WITH STEEL SURFACES.
3. CLEAN ARCHITECTURALLY EXPOSED MEMBERS PER SSPC_SP-10-63 AND PRIME WITH A ZINC-RICH PRIMER.
4. SEE ARCHITECTURAL DRAWINGS FOR FINAL PAINTING.

POST INSTALLED ANCHORS:

1. POST-INSTALLED ANCHORS SHALL BE USED ONLY WHERE SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS.
2. CONTRACTOR SHALL OBTAIN APPROVAL FROM ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
3. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH OR DAMAGE TO EXISTING REBAR.
4. HOLES SHALL BE DRILLED AND CLEANED AND ANCHORS SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER PRINTED INSTALLATION INSTRUCTIONS (MPII). ANCHORS SHALL BE INSTALLED AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACING INDICATED IN THE MPII.

SHOP-DRAWINGS

1. CONTRACTOR SHALL SUBMIT TO THE ARCHITECT COMPLETE SHOP-DRAWINGS FOR BUILDING COMPONENTS NOT DESIGNED BY THE DESIGN TEAM OF RECORD PRIOR TO FABRICATION AND INSTALLATION. SHOP-DRAWINGS REQUIRING ENGINEERING DESIGN SHALL BE SIGNED AND STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF GEORGIA.
2. SIGNED AND STAMPED SHOP-DRAWINGS SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.
3. REVIEW OF SHOP-DRAWINGS BY THE DESIGN TEAM OF RECORD DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FOR ERRORS AND OMISSIONS WITHIN THE SHOP-DRAWINGS. REVIEW OF SHOP-DRAWINGS BY THE ARCHITECT OR ENGINEER DOES NOT CONSTITUTE A CHANGE TO THE CONTRACT. WHERE DISCREPANCIES EXIST BETWEEN THE CONTRACT DOCUMENTS AND THE SHOP-DRWINGS THE CONTRACT DOCUMENTS SHALL TAKE PRECEDENCE UNLESS THE CHANGE IS EXPLICITLY AUTHORIZED IN WRITING.

CONCRETE MASONRY NOTES

1. ALL MASONRY DESIGN AND CONSTRUCTION SHALL CONFORM TO ACI 530 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AND ACI 530.1 SPECIFICATION FOR MASONRY STRUCTURES.
2. 28-DAY STRENGTH OF MASONRY f'm SHALL BE 2000 PSI MINIMUM.
3. MASONRY UNITS SHALL CONFORM TO ASTM C-90.
4. GROUT SHALL CONFORM TO ASTM C476. GROUT STRENGTH SHALL BE 3000 PSI MINIMUM.
5. C.M.U. SHALL BE PLACED IN RUNNING BOND, UNLESS NOTED OTHERWISE.
6. PROVIDE FULL MORTAR BEDDING. MORTAR SHALL BE TYPE M OR S.
7. MASONRY WALLS SHALL BE TEMPORARILY BRACED UNTIL ALL SUPPORTING ELEMENTS AND CONNECTIONS ARE IS IN PLACE AND CONCRETE SLABS HAVE REACHED DESIGN STRENGTH. IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO PROVIDE AND INSTALL BRACING AND SHORING FOR ALL MASONRY WALLS AS REQUIRED TO ENSURE STABILITY DURING CONSTRUCTION.
8. PROVIDE 9-GAUGE LADDER-TYPE HORIZONTAL REINFORCEMENT AT 16" O.C. VERTICALLY FOR FULL HEIGHT OF WALLS, UNLESS NOTED OTHERWISE. LAP LADDER REINFORCEMENT 8 INCHES MINIMUM AT SPLICES. LAP AT WALL INTERSECTIONS AND CORNERS. HORIZONTAL REINFORCEMENT SHALL BE DISCONTINUOUS AT CONTRACTION JOINTS. SEE SPECS FOR JOINT REINFORCEMENT DETAILS.
9. CONNECT C.M.U. WALLS TO PRE-ENGINEERED STEEL COLUMNS AT 16" VERTICALLY. SEE ARCHITECTURAL DRAWINGS FOR CONNECTION DETAILS.
10. PROVIDE C.M.U. LINTELS ABOVE AND BELOW ALL OPENINGS THROUGH CMU WALLS. SEE CMU LINTEL SCHEDULE AND NOTES ON THIS SHEET FOR LINTEL DETAILS AND FOR REINFORCED CELLS AT LINTEL BEARING LOCATIONS AT EACH SIDE OF OPENINGS. SEE ARCHITECTURAL DRAWINGS FOR ALL CMU WALL OPENING SIZES AND LOCATIONS.
11. MAXIMUM CONTROL JOINT SPACING IN MASONRY WALLS = 30'-0" UNLESS NOTED OTHERWISE. SEE ARCH DRAWINGS FOR JOINT LOCATIONS.
12. UNLESS NOTED OTHERWISE, CMU BOND-BEAMS SHALL BE PROVIDED AT BUILDING FLOOR AND ROOF ELEVATIONS, STAIR LANDINGS, AND ALONG TOPS OF ALL WALLS. UNLESS NOTED OTHERWISE, ALL U-BLOCKS/BOND BEAMS ON STRUCTURAL AND ARCHITECTURAL DRAWINGS SHALL BE FILLED WITH 3000 PSI GROUT AND REINFORCED WITH 2#5 BARS CONTINUOUS (BOTTOM). PROVIDE CORNER BARS AT CORNERS, LAPPING 48 BAR DIAMETERS WITH HORIZONTAL BARS. BARS SHALL BE CONTINUOUS AT CONTRACTION JOINTS.
13. ALL C.M.U. WALLS ON STRUCTURAL AND ARCHITECTURAL DRAWINGS SHALL BE REINFORCED WITH VERTICAL REBAR AT EACH CORNER, AT EACH SIDE OF OPENINGS, AT EACH SIDE OF COLUMNS AND CONTRACTION JOINTS, AND AT MAXIMUM SPACINGS INDICATED IN THESE STRUCTURAL DRAWINGS. FILL ALL REINFORCED CELLS OF BLOCK VERTICALLY WITH 3000 PSI GROUT FROM FOUNDATION TO TOP OF WALL. PROVIDE DOWELS OF MATCHING SIZE WITH STANDARD HOOK IN FOUNDATIONS.
14. FILL ALL BELOW-GRADE CELLS WITH GROUT.
15. MINIMUM LAPS OF REINFORCING BARS SHALL BE AS FOLLOWS:

#4 BARS	2'-4"
#5 BARS	3'-0"
#6 BARS	3'-7"

16. MASONRY SHALL BE PROTECTED FROM FREEZING DURING PLACEMENT & CURING. COLD WEATHER MASONRY PROCEDURES SHALL COMPLY W/ THE ABOVE REFERENCED STANDARDS.
17. SEE ALL CMU MASONRY DETAILS IN THESE DRAWINGS.
18. CMU WALL REINFORCING SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION. DRAWINGS SHALL SHOW ALL WALL AND PILASTER REINFORCING IN PLAN AND IN ELEVATION.

C.M.U. LINTEL SCHEDULE:

CLEAR SPAN	LINTEL DEPTH	REINFORCEMENT
0'-0" - 3'-6"	8"	2 #4
3'-6" - 5'-0"	8"	2 #5
5'-0" - 7'-0"	8"	2 #6

C.M.U. LINTEL NOTES

1. AT EACH END BEAR C.M.U. LINTELS 2'-0" MINIMUM ON REINFORCED WALL.
2. UNLESS NOTED OTHERWISE, PROVIDE FULL HEIGHT REINFORCED AND GROUTED CELLS AT EACH SIDE OF OPENINGS. UNLESS NOTED OTHERWISE, PROVIDE 2 REINFORCED CELLS EACH SIDE OF OPENINGS EQUAL TO AND LESS THAN 6'-0" WIDE, AND 3 REINFORCED CELLS EACH SIDE OF OPENINGS GREATER THAN 6'-0" WIDE. BAR SIZES SHALL BE AS INDICATED IN STRUCTURAL SECTIONS. HOOK VERTICAL BAR IN BOND BEAM AT TOP. PROVIDE MATCHING DOWEL HOOKED IN FOUNDATION.
3. SCHEDULED LINTELS SHALL BE PROVIDED ABOVE AND BELOW ALL WALL PENETRATIONS, UNLESS NOTED OTHERWISE.

PRE-ENGINEERED METAL BUILDING NOTES

1. THE DESIGN, FABRICATION, AND ERECTION OF THE PRE-ENGINEERED BUILDING SHALL CONFORM TO IBC 2024. DESIGN LOADS SHALL BE IN ACCORDANCE WITH ASCE 7-16.
2. PRE-ENGINEERED METAL BUILDING MANUFACTURER SHALL PROVIDE SHOP-DRAWINGS AND CALCULATIONS SEALED AND SIGNED BY A REGISTERED PROFESSIONAL STRUCTURAL ENGINEER IN THE STATE OF GEORGIA. SHOP-DRAWINGS AND CALCULATIONS SHALL SHOW WIND AND SEISMIC DESIGN DATA IN ACCORDANCE WITH IBC 2018 AND ASCE 7-16. SHOP-DRAWINGS AND CALCULATIONS SHALL SHOW REACTIONS AT CONNECTIONS TO THE FOUNDATION, INCLUDING DEAD AND LIVE LOADS, LATERAL THRUST, UPLIFT, WIND, AND SEISMIC LOADS. SHOP-DRAWINGS SHALL INCLUDE ANCHOR BOLT DETAILS AND LAYOUT. SHOP-DRAWINGS SHALL INDICATE THAT BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RCSC-2000.
3. THE PRE-ENGINEERED METAL BUILDING SHALL NOT TRANSFER MOMENTS TO THE FOUNDATION.
4. LATERAL DRIFT DUE TO WIND LOADS OF PRE-ENGINEERED BUILDINGS WITH ATTACHED MASONRY SHALL NOT EXCEED H/200.
5. THE ANCHOR BOLT TYPE AND DIAMETERS SHALL BE DESIGNED AND SPECIFIED BY THE PRE-ENGINEERED METAL BUILDING DESIGNER. SEE STRUCTURAL DETAILS.
6. THE PRE-ENGINEERED METAL BUILDING SHALL BE DESIGNED TO SUPPORT THE LOADS IMPOSED ON THE STRUCTURE BY MECHANICAL, ELECTRICAL, PLUMBING, AND OTHER EQUIPMENT SHOWN ON THE CONTRACT DOCUMENT DRAWINGS AND SPECS.
7. ALL COLUMNS SHALL BE LOCATED ON GRIDLINES AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
8. SEE ARCHITECTURAL DRAWINGS FOR METAL BUILDING COLUMN DIMENSION CRITERIA AT CONCRETE-ENCASED COLUMNS AT TRUCK PARKING AREA.
9. PEMB SHALL BE DESIGNED FOR ALL DOORS AND ROLL-UP DOORS IN BOTH THE OPEN AND CLOSED CONDITION DURING FULL DESIGN WIND SPEED EVENT.
10. UNLESS NOTED OTHERWISE, AT OVERHEAD DOORS PEMB SUPPLIER SHALL PROVIDE STRUCTURAL CHANNEL JAMBS AND HEADERS, RATHER THAN COLD-FORMED JAMBS & HEADERS. AT ALL OVERHEAD DOORS PEMB SUPPLIER SHALL PROVIDE ADDITIONAL GIRTS DESIGNED AND LOCATED FOR INSTALLATION OF OVERHEAD DOORS.

COMPONENTS & CLADDING WIND PRESSURES DIAGRAMS AND SCHEDULES:

ALL COMPONENTS & CLADDING PRESSURES ARE GIVEN AS ULTIMATE & SERVICE LEVEL PRESSURES PER ASCE 7-10

DIAGRAM OF BLDG ENVELOPE

COMPONENT & CLADDING DESIGN WIND PRESSURES FOR EXTERIOR WALLS (ULTIMATE/SERVICE) (PSF)					
LOCATION		EFFECTIVE WIND AREA			
		10 FT^2	50 FT^2	100 FT^2	200 FT^2
NEGATIVE	ZONE 4, p_6	−42.9 / −29.0	−39.8 / −26.9	−38.5 / −23.1	−37.2 / −22.3
	ZONE 5, p_6	−50.5 / −34.1	−44.3 / −26.6	−41.6 / −25.0	−38.9 / −23.3
POSITIVE	ZONE 4 & 5, p_5	+40.4 / +27.3	+37.3 / +22.4	+36.0 / +21.6	+34.7 / +20.8

NOTES:
1. POSITIVE PRESSURE DENOTES WIND TOWARD WALL SURFACE, AND NEGATIVE PRESSURE DENOTES WIND AWAY FROM WALL SURFACE.

DIAGRAM OF ROOF PLAN

COMPONENT & CLADDING GROSS DESIGN WIND PRESSURES ALONG ROOF (ULTIMATE/SERVICE) (PSF)				
LOCATION		EFFECTIVE WIND AREA		
		10 FT^2	50 FT^2	100 FT^2
NEGATIVE	ZONE 1	−42.3 / −25.4	−41.3 / −24.8	−40.4 / −24.2
	ZONE 2	−65.5 / −39.3	−51.9 / −31.1	−46.0 / −27.6
	ZONE 3	−93.4 / −56.0	−60.3 / −36.2	−46.0 / −27.6
POSITIVE	ZONE 1	+23.7 / +14.2	+21.7 / +13.0	+20.9 / +12.5
	ZONES 2 & 3	+23.7 / +14.2	+21.7 / +13.0	+20.9 / +12.5
OVERHANG	ZONE 2	−47.4 / −28.4	−45.4 / −27.2	−44.6 / −26.8
	ZONE 3	−78.0 / −46.8	−39.1 / −23.5	−22.3 / −13.4

NOTES:
1. POSITIVE PRESSURE DENOTES WIND TOWARD ROOF SURFACE, AND NEGATIVE PRESSURE DENOTES WIND AWAY FROM ROOF SURFACE.

PROJECT NUMBER
2512

DATE
03/12/26

DRAWN BY
MJM

APPROVED BY
MJM

REVISIONS
2 4/15/26 - ADDENDUM 2

CITY OF SOCIAL CIRCLE
PUBLIC WORKS DEPARTMENT
New Fleet Facility
New Vine Circle, Social Circle, GA 30025

TURNIPSEED ENGINEERS
ATLANTA
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No. SE000863
MARCUS J. MILLARD

SHEET NUMBER
S0.01

COLUMN FOOTING SCHEDULE:

MARK	SIZE	REINF.	NOTES
F4	4'-0"x4'-0"x24"	#8 5 EA WAY TOP & BOTT	
F6	6'-0"x6'-0"x24"	5#5 EA WAY TOP T#5 EA WAY BOTT	
F7	7'-0"x7'-0"x24"	5#6 EA WAY TOP T#6 EA WAY BOTT	

A

COLUMN FOOTING SCHEDULE NOTES:

1. SEE DETAIL 3/S3.02 FOR FOOTINGS AT PRE-ENGINEERED METAL BUILDING COLUMNS.
2. SEE DETAILS 3/S3.02 AND 6/S3.02 AT CONCRETE-ENCASED PRE-ENGINEERED METAL BUILDING COLUMNS.
3. SEE 5/S3.02 FOR CONCRETE PIERS CAST MONOLITHICALLY WITH SLAB ON EACH COLUMN FOOTING.

FOUNDATION PLAN LEGEND:

F_J_ : INDICATES SAW-CUT FLOOR JOINT. SEE 4I5/S3.01 FOR DETAILS. REINFORCEMENT SHALL BE DISCONTINUOUS AT THE JOINTS. FILL ALL JOINTS WITH AN ELASTOMERIC JOINT FILLER/SEALANT AFTER CONCRETE CURES FOR 28 DAYS MINIMUM.

P1, P2_ : INDICATES CAST-IN-PLACE CONCRETE PIER POURED MONOLITHIC WITH THE SLAB. SEE 5/S3.02 FOR PIER REINFORCEMENT, DIMENSIONS, AND OTHER DETAILS.

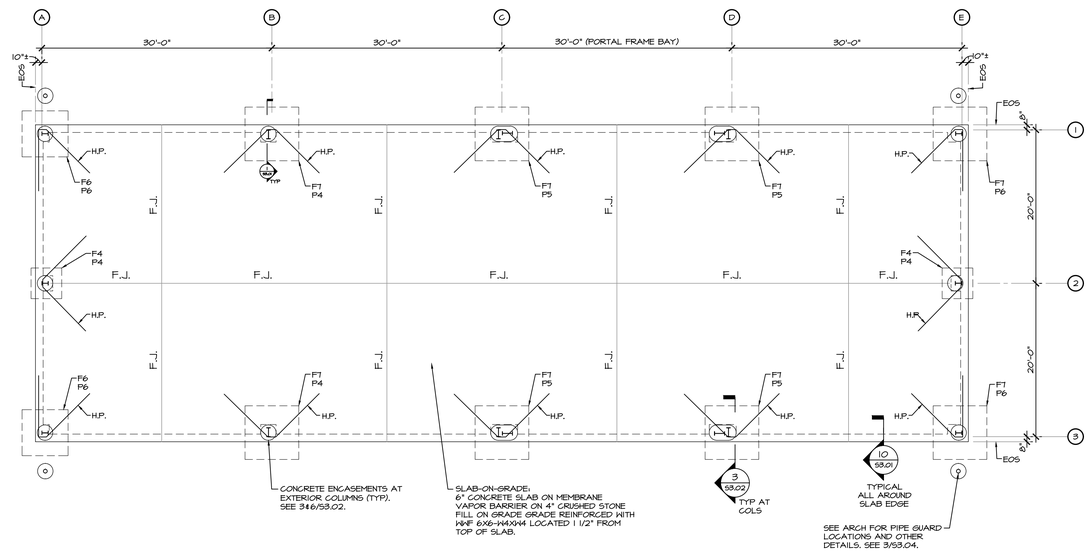
H.P. : INDICATES #5x16'-0" HAIRPIN REBAR WRAPPED AROUND COLUMN ANCHOR BOLTS. SEE COLUMN DETAILS.

TOF : INDICATES TOP OF FOOTING ELEVATION REFERENCED FROM FINISH FLOOR ELEVATION 0'-0". SEE NOTE #3 ON THIS SHEET.

EOS : INDICATES EDGE OF SLAB ON GRADE

FOUNDATION PLAN NOTES:

1. SEE ARCHITECTURAL DRAWINGS FOR FOR EXACT LOCATIONS OF FLOOR SLOPES AND ADDITIONAL DIMENSIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION.
2. SEE CIVIL DRAWINGS FOR TYPICAL TOP OF SLAB-ON-GRADE. SEE ARCH FOR SLAB SLOPES.
3. TYPICAL TOP-OF-FOOTING ELEVATION AT PRE-ENGINEERED METAL BUILDING COLUMNS SHALL BE -2'-0" BELOW FF, UNLESS NOTED OTHERWISE.
4. MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 4,500 PSI FOR SLABS-ON-GRADE AND 3,000 PSI FOR WALL FOOTINGS AND COLUMN FOOTINGS. SEE CONCRETE NOTES ON S0.01 FOR OTHER CONCRETE DETAILS.
5. CONTRACTOR COORDINATE FOOTING AND ANCHOR BOLT LAYOUT WITH FINAL METAL BUILDING DESIGN. FOOTINGS SHALL BE CENTERED UNDER THE COLUMN BASE PLATES, WHERE FOOTING SUPPORTS MAIN FRAME COLUMN AND PORTAL COLUMN, FOOTING SHALL BE CENTERED UNDER THE MAIN FRAME COLUMN. VERIFY ALL BASE PLATE LOCATIONS PRIOR TO POURING CONCRETE. IN THE EVENT OF ANY DISCREPANCIES BETWEEN THE FINAL METAL BUILDING DESIGN AND THESE CONSTRUCTION DOCUMENTS NOTIFY THE ARCHITECT.
6. SEE 3/S3.02 AND 6/S3.02 FOR METAL BUILDING COLUMN DIMENSION CRITERIA AT CONCRETE-ENCASED COLUMNS AT TRUCK PARKING.



1 S2.01 FOUNDATION PLAN -COVERED EQUIPMENT STORAGE

SCALE: 1/8 = 1'-0"

PROJECT NUMBER
2512
DATE
03/12/26
DRAWN BY
MJM
APPROVED BY
MJM

REVISIONS
A 4/15/26 - ADDENDUM 2

CITY OF SOCIAL CIRCLE
PUBLIC WORKS DEPARTMENT
New Fleet Facility
New Vine Circle, Social Circle, GA 30025

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Millard, Inc. ARCHITECTS ENGINEERS

GEORGIA REGISTERED No. SE000863 STRUCTURAL ENGINEER MARCUS J. MILLARD

SHEET NUMBER

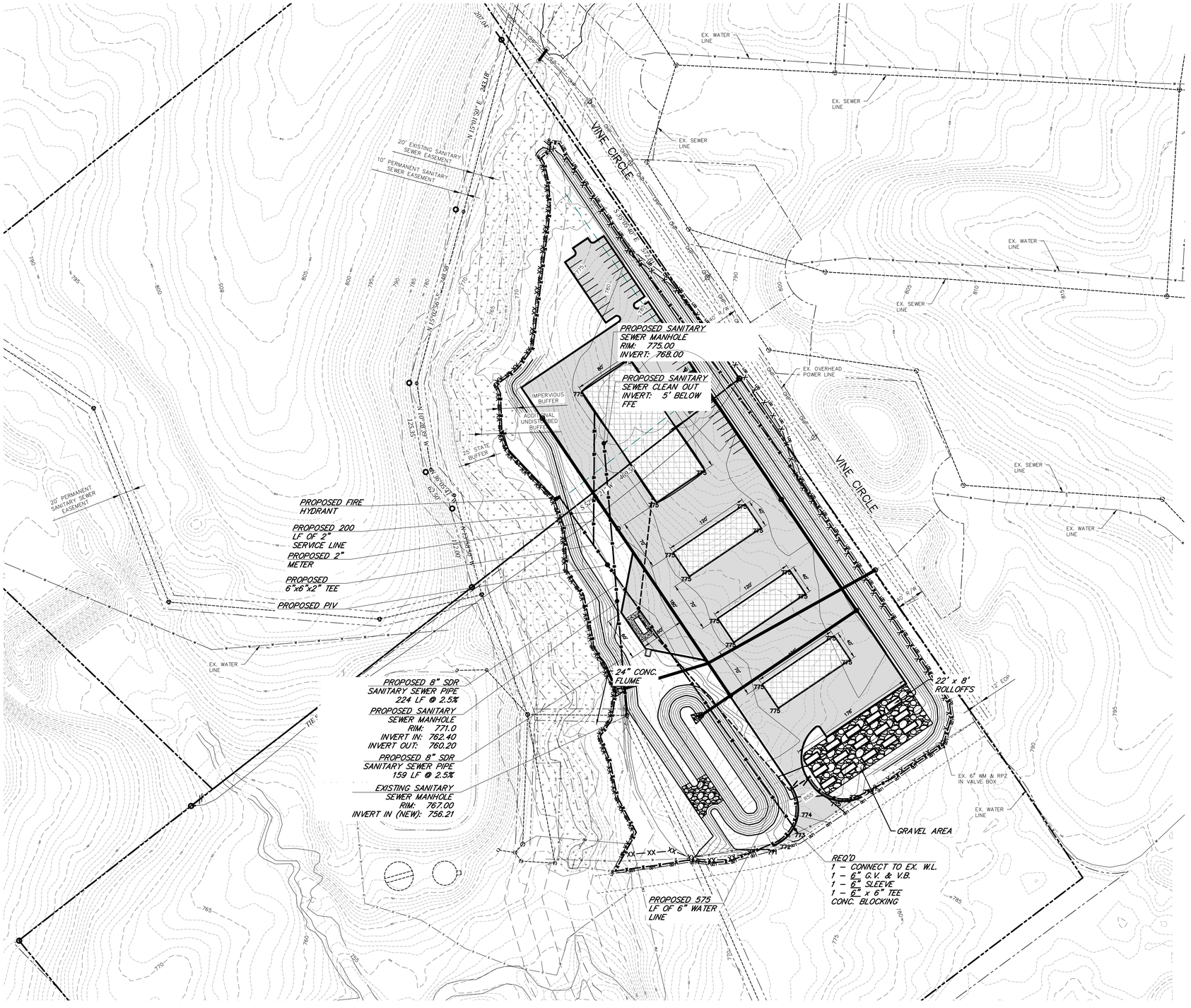
S2.01

UTILITY NOTES:

1. SEE SHEET 2 FOR PROJECT NOTES.
2. SEE SHEET 3 FOR BENCH MARK ELEVATIONS.
3. EXISTING WATER SERVICE SHALL BE VERIFIED AS COPPER FROM THE TAP TO THE METER, IF NOT THEN THE SERVICE LINE WILL NEED TO BE UPGRADED.
4. THE PROPOSED FIRE LINE SHALL BE RJ DIP FROM THE TAP TO THE BACK FLOW DEVICE.
5. INSTALL TRAFFIC RATED COLLAR AND CAP ON ALL CLEANOUTS IN PAVEMENT OR SIDEWALKS.
6. NO TREE SHALL BE PLANTED IN COUNTY OWNED UTILITY EASEMENTS.
7. NO SHRUBS SHALL BE PLANTED ON TOP OF WATER LINES/ SERVICES AND SANITARY SEWER LINES/ SERVICES.
8. IF ANY AND ALL EXTRA WATER SERVICE TAPS THAT WILL NOT BE UTILIZED SHALL BE ABANDONED AT THE CORPORATION.

LEGEND

	EXISTING	PROPOSED
STRUCTURE		
ROADWAY		
GRAVEL ROLLOFF		
FENCE		
PROPERTY LINE		
MINIMUM BUILDING LINE		
BUFFER		
CONTOUR	680	680
STORM SEWER LINE		
SEWER LINE & SERV.		
FORCE MAIN		
FIRE PROTECTION		
WOODS LINE		
DRAINAGE DITCH		
CONSTRUCTION LIMITS		LOD
FLOW DIRECTION		



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2512
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ADDENDUM 2 04/16/26

CITY OF SOCIAL CIRCLE
PUBLIC WORKS DEPARTMENT
WATER & SEWER UTILITY PLAN
NEW VINE CIRCLE, SOCIAL CIRCLE, GEORGIA 30025

TURNIPSEED ENGINEERS
ATLANTA
ANGUSTA
AVON
ST. SIMONS ISLAND

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Architects & Engineers
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Roswell, Georgia 30075
770-993-2034

Millard, Inc. ARCHITECTS ENGINEERS

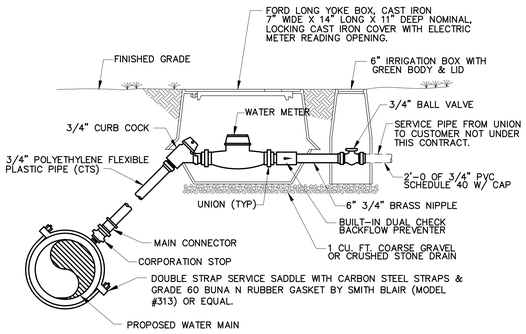
GEORGIA REGISTERED No. 36129 PROFESSIONAL ENGINEER WILLIAM D. MARTIN

GSWCC LEVEL II CERTIFICATION NO. 000055308

SHEET NUMBER

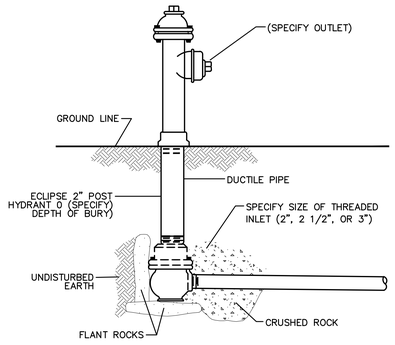
E-6

G:\Social Circle\242774 Social Circle Public Works Facility\04.2 - Drawings\4 - Construction Drawings\242774_Base DWF_addendum_2.dwg



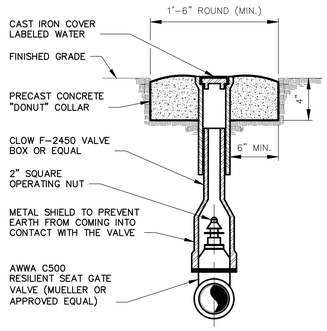
TYPICAL SERVICE CONNECTION

N.T.S.



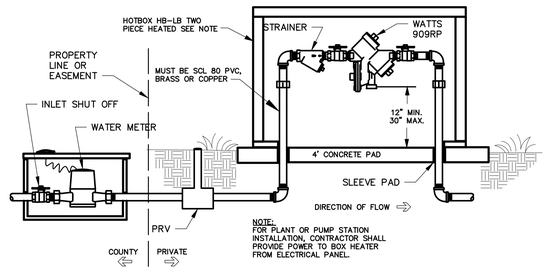
HYDRANT DETAIL

N.T.S.



VALVE BOX INSTALLATION DETAIL

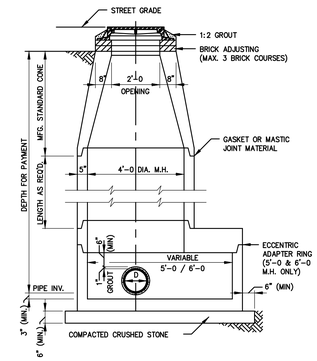
N.T.S.



BACKFLOW PREVENTER OUTDOOR HORIZONTAL INSTALLATION
2.5" AND SMALLER

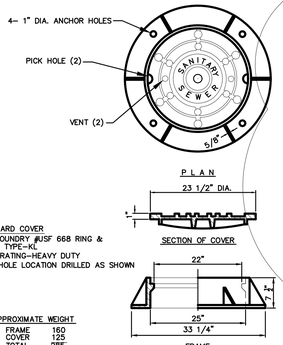
N.T.S.

NOTE:
M.H. COVERS TO BE SET AT GRADE IN ROADWAYS, ROAD SHOULDERS AND RESIDENTIAL YARDS. ALL OTHER M.H. COVERS TO EXTEND 1'-0" ABOVE GRADE EXCEPT WHERE NOTED OTHERWISE ON CONTRACT PLANS.



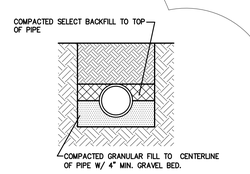
TYP. PRECAST CONC. M.H. DET.

N.T.S.



VENTED MANHOLE FRAME & COVER

N.T.S.



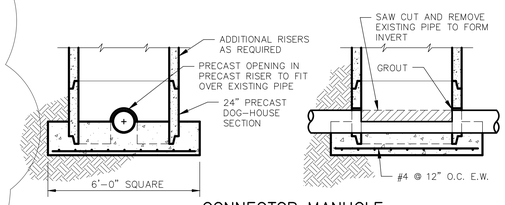
TYPE 5

N.T.S.

PIPE SIZE IN	PRESSURE CLASS	NOMINAL THICK IN	TYPE 5 TRENCH
8	350	0.25	50
10	350	0.26	45
12	350	0.28	44
14	250	0.28	36
16	250	0.30	34
18	250	0.31	31
20	250	0.33	30
24	200	0.33	25
30	150	0.34	22
36	150	0.38	21
42	150	0.41	20
48	150	0.46	20

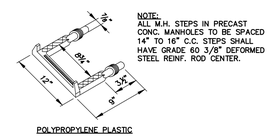
SEWER PIPE BEDDING DETAIL

N.T.S.



CONNECTOR MANHOLE

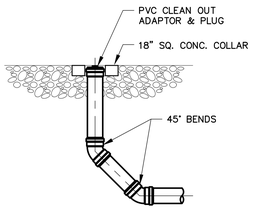
N.T.S.



NOTE:
ALL M.H. STEPS IN PRECAST CONC. MANHOLES TO BE SPACED 14" TO 16" C.C. STEPS SHALL HAVE GRADE 60 3/8" DEFORMED STEEL REINF. ROD CENTER.

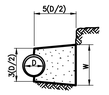
MANHOLE STEP DETAILS

N.T.S.



CLEANOUT DETAIL

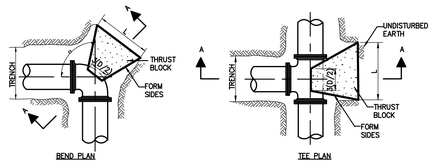
N.T.S



PIPE SIZE	TEES & PLUGS W	TEES & PLUGS L	BENDS 0=22-1/2° W	BENDS 0=22-1/2° L	BENDS 0=45° W	BENDS 0=45° L	BENDS 0=90° W	BENDS 0=90° L
4"	1'-0"	1'-3"	8"	9"	10"	1'-2"	1'-2"	1'-7"
6"	1'-6"	1'-11"	1'-0"	1'-2"	1'-2"	1'-11"	1'-8"	2'-5"
8"	2'-0"	2'-7"	1'-2"	1'-6"	1'-8"	2'-7"	2'-2"	3'-4"
10"	2'-6"	3'-2"	1'-4"	2'-4"	1'-10"	3'-4"	2'-8"	4'-2"
12"	3'-0"	3'-10"	1'-8"	2'-8"	2'-2"	4'-0"	3'-2"	5'-1"
16"	4'-0"	5'-1"	2'-3"	3'-5"	2'-11"	5'-4"	4'-2"	6'-10"
24"	5'-11"	7'-8"	3'-5"	5'-2"	4'-4"	8'-0"	5'-0"	12'-9"

CONCRETE BLOCK DIMENSIONS

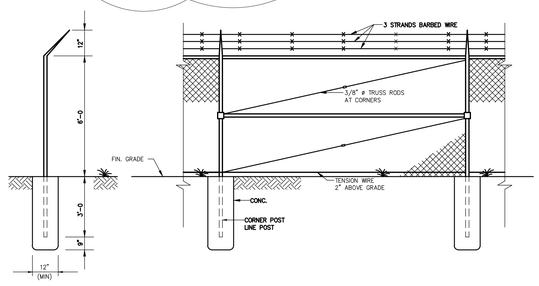
NOTE:
COST OF CONCRETE BLOCKING TO BE INCLUDED IN COST OF FITTINGS, VALVES, AND FIRE HYDRANTS AS REQUIRED. NO SEPARATE PAYMENT WILL BE MADE FOR CONCRETE BLOCKING.



NOTES:
1. THRUST BLOCKING TO BE CLASS "B" CONCRETE.
2. THESE DIMENSIONS WERE CALCULATED USING A SOIL BEARING CAPACITY OF 1500 PSF IF CAPACITY IS FOUND TO BE LESS THAN 1500 PSF THE DIMENSIONS SHALL BE REVISED ACCORDING TO THE ENGINEER.
3. DIMENSIONS TO BE STRICTLY ADHERED TO.
4. BLOCKS SHALL BEAR AGAINST UNDISTURBED SOIL.
5. DESIGN PRESSURE 4" THRU 24" 150 P.S.I.
6. ALL BOLTS TO REMAIN ACCESSIBLE. DO NOT COVER WITH CONCRETE.

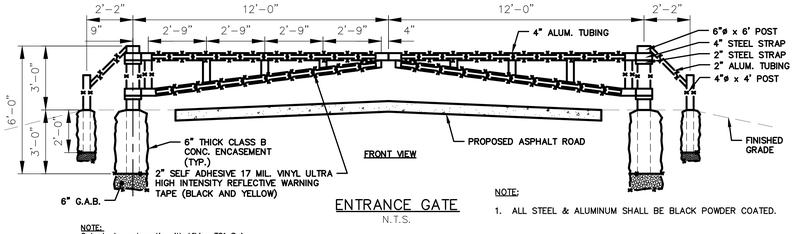
CONCRETE BLOCKING DETAILS

N.T.S.



CHAIN LINK FENCE DETAIL

N.T.S.



ENTRANCE GATE

N.T.S.

NOTE:
Gate to be automatic with Viking T21 Swing Gate Operator (one per side) with the following: Complete Gate Control Kit, Viking Monitor, Keypad, Pedestal, Protective hood for Keypad, Automatic Free Exit System, Security Brands Access Box, SOS Emergency Access Siren, Solar Package kit, and Outdoor Maglock Kit.

NOTE:
1. ALL STEEL & ALUMINUM SHALL BE BLACK POWDER COATED.

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ADDENDUM 1 04/07/2026
ADDENDUM 2 04/16/2026

CITY OF SOCIAL CIRCLE
PUBLIC WORKS DEPARTMENT
MISCELLANEOUS DETAILS
NEW VINE CIRCLE, SOCIAL CIRCLE, GEORGIA 30025

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GEORGIA REGISTERED No. 36129 PROFESSIONAL ENGINEER WILLIAM D. MARTIN

GSWCC LEVEL II
CERTIFICATION NO. 000055308

SHEET NUMBER

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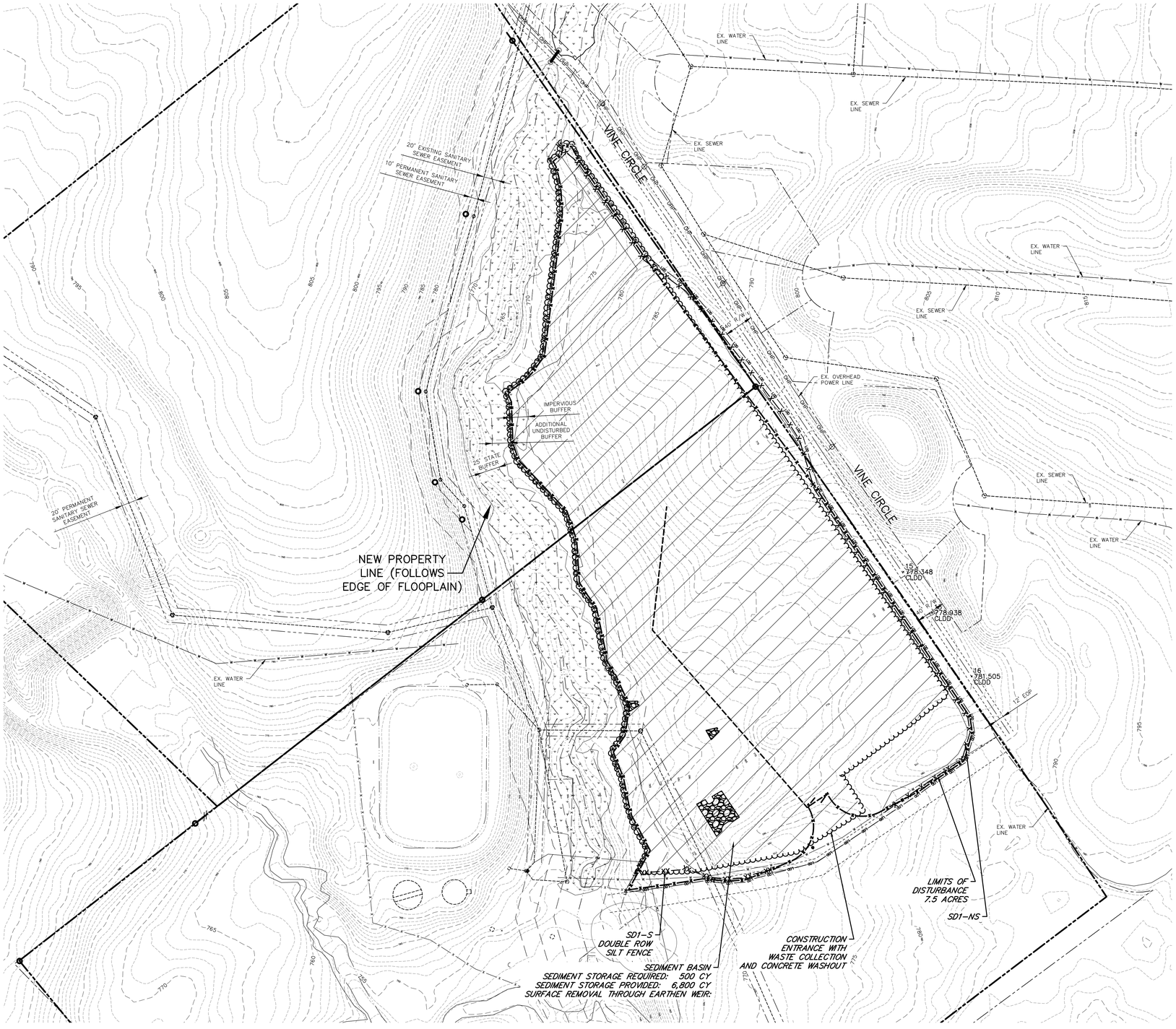

SOIL EROSION & SEDIMENT CONTROL NOTES:

1. THE ESCAPE OF SEDIMENT FROM THE SITE SHALL BE PREVENTED BY THE INSTALLATION OF EROSION & SEDIMENT CONTROL MEASURES & PRACTICES PRIOR TO, OR CONCURRENT WITH, L& DISTURBING ACTIVITIES.
2. EROSION CONTROL MEASURES WILL BE MAINTAINED AT ALL TIMES, IF FULL IMPLEMENTATION OF THE APPROVED PLAN DOES NOT PROVIDE FOR EFFECTIVE EROSION CONTROL, ADDITIONAL, EROSION & SEDIMENT CONTROL MEASURES SHALL BE IMPLEMENTED TO CONTROL OR TREAT THE SEDIMENT SOURCE.
3. ANY DISTURBED AREA LEFT EXPOSED FOR A PERIOD GREATER THAN 14 DAYS SHALL BE STABILIZED WITH MULCH OR TEMPORARY SEEDING.
4. ANY SLOPES 3:1 & GREATER SHALL BE COVERED WITH EROSION CONTROL MATTING &/OR BLANKETS.
5. PRIOR TO REMOVING BMP'S, CONTRACTOR SHALL MEET ONSITE WITH THE COUNTY TO ENSURE FINAL STABILIZATION IS COMPLETE.
6. PRIOR TO FILING AN NPDES NOTICE OF TERMINATION, THE CONTRACTOR SHALL MEET ONSITE WITH THE COUNTY TO ENSURE THAT ALL SILT FENCE & TEMPORARY BMP'S HAVE BEEN REMOVED.
7. NON-EXEMPT ACTIVITIES SHALL NOT BE CONDUCTED WITHIN THE 25 OR 50 FOOT UNDISTURBED STREAM BUFFERS AS MEASURED FROM THE POINT OF WRESTED VEGETATION OR WITHIN 25 FEET OF THE COASTAL MARSH L& BUFFER AS MEASURED FROM THE JURISDICTIONAL LINE WITHOUT FIRST ACQUIRING THE NECESSARY VARIANCES & PERMITS. SEE NOTE 15 ON SHEET 5.

LEGEND

	EXISTING	PROPOSED
STRUCTURE		
ROADWAY		
ROADWAY		
FENCE		
PROPERTY LINE		
MINIMUM BUILDING LINE		
BUFFER		
CONTOUR	680	680
STORM SEWER LINE		
SEWER LINE & SERV.		
FORCE MAIN		
FIRE PROTECTION		
WOODS LINE		
DRAINAGE DITCH		
CONSTRUCTION LIMITS		LOD
FLOW DIRECTION		

SOIL MAP

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Alp	Chewacla, Cartecay, and Toccoa soils, 0 to 2 percent slopes, frequently flooded	1.0	9.9%
AnD3	Appling sandy clay loam, 10 to 15 percent slopes, severely eroded	1.7	17.5%
CdB2	Cecil coarse sandy loam, 2 to 6 percent slopes, eroded	0.0	0.4%
CZB3	Cecil sandy clay loam, 2 to 6 percent slopes, severely eroded	1.6	16.3%
CZC3	Cecil sandy clay loam, 6 to 10 percent slopes, severely eroded	4.7	48.4%
CZD3	Cecil sandy clay loam, 10 to 15 percent slopes, severely eroded	0.5	5.1%
CZD4	Cecil-Gullied land complex, 10 to 15 percent slopes	0.2	2.4%
Totals for Area of Interest		9.8	100.0%

SOIL LEGEND

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PUBLIC WORKS DEPARTMENT
INITIAL SESC PLAN
NEW VINE CIRCLE, SOCIAL CIRCLE, GEORGIA 30025

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GSWCC LEVEL II CERTIFICATION NO. 0000555308

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